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TOWN OF LEWISBORO Westchester County, New York



Tel: (914) 763-5592 Fax: (914) 875-9148 Email: planning@lewisborogov.com

Planning Board 79 Bouton Road South Salem, New York 10590

AGENDA

Tuesday, February 28, 2023

Meeting will start at 7:30 p.m. and end at or before 11:00 p.m.

I. EXTENSION OF TIME REQUESTS

Cal #08-14PB, Cal# 95-14WP, Cal# 20-14SW

Goldens Bridge Village Center, NYS Route 22, Goldens Bridge, NY 10526; Sheet 4, Block 11126, Lot 07 (**Stephen Cipes, owner of record**) – The Planning Board Resolution for Site Development Plan Approval, Wetland Activity Permit Approval and Town Stormwater Permit granted on January 21, 2020 for modifications to the existing shopping center expired January 21, 2022.

Cal #04-19PB, Cal #17-19WP, Cal #06-19SW

Pound Ridge Stone, 2 West Road, South Salem, NY 10590; Sheet 49B, Block 9831, Lot 1 (Two West Road LLC, owner of record) – The Planning Board Resolution for a Negative Declaration of Significance, Site Development Plan Approval, Special Use Permit Approval, Town Wetland Activity Permit Approval and Town Stormwater Permit Approval granted on August 17, 2021 for site upgrades including additional parking and storage areas expired February 17, 2023.

II. PUBLIC HEARINGS

Cal #06-22PB, Cal #05-22WP, Cal #03-22SW

Waccabuc Country Club Snack Bar, 18 Perch Bay Road, Waccabuc, NY 10597; Sheet 25A, Block 10813, Lot 1 & 0 Tarry-A-Bit Lane, Waccabuc, NY 10597; Sheet 25, Block 11155, Lot 148 (Waccabuc Country Club Co., owner of record for both lots) - Application for beachfront improvements including renovation of the boathouse, construction of a pavilion, replacement of the snack bar, and installation of accessible parking and walkways.

Cal #02-20PB

Mandia Residences, 65 Old Bedford Road, Goldens Bridge, NY 10526; Sheet 4A, Block 11112, Lot 2 (Town of Lewisboro, owner of record) - The Planning Board Resolution for Site Development Plan Approval, Special Use Permit Approval and Town Stormwater Permit Approval granted on June 19, 2020 included a construction performance bond for the four apartments and a request for a partial bond release has been submitted.

III. DECISION

Cal #10-17PB, Cal #19-22SW

Mercedes Benz of Goldens Bridge, 321 Main Street, Goldens Bridge, NY 10526; Sheet 4E, Block 11135, Lots 1, 2, 3, 4, 5, 6, 7 & 9 and Sheet 4E, Block 11137, Lot 42 (Celebrity Westchester Realty, LLC., owner of record for the nine lots) – Based on the applicant's January 17, 2023 request for an amendment of the Approving Site Development Plan Approval and Town Stormwater Permit Approval Resolution (dated March 17, 2020) and Amended Resolution (dated January 17, 2023) for installation of a water treatment system.

IV. SUBDIVISION

Cal #15-22PB

Vandervoort/Rising Starr Subdivision, 93 Silver Spring Rd, Wilton, CT 06897; Sheet 48, Block 10057, Lots 14, 19, 84 & 134 (SJK, LLC & PVK, LLC, owners of record) - Application for a subdivision.

V. SITE DEVELOPMENT PLAN REVIEW

Cal #18-22PB

Bichon LLC, 876 Route 35, Cross River, NY 10518; Sheet 20, Block 10801, Lot 2 (Bichon LLC – owner of record) – Application for a change of use from residential to commercial (professional office and outdoor storage of containers).

Courtroom at 79 Bouton Road

VI. WETLAND PERMIT REVIEW

Cal #34-22WP, Cal #01-21WV

Maple Tree Farm, 400 Smith Ridge Road, South Salem, NY 10590; Sheet 24, Block 9831, Lot 49B (Maple Tree Farm, LLC, owner of record) - Application for remediation of wetlands.

Cal #39-22WP

Morrissey Residence, 10 Hoyt Street, South Salem, NY 10590; Sheet 36C, Block 11172, Lot 5 (Susan Morrissey, owner of record) - Application for sunroom/porch, terrace and walkway.

Cal #42-22WP and Cal #21-22SW

Simpkins Residence, 120 Mill River Road, South Salem, NY 10590; Sheet 45, Block 10299, Lot 86 (Nancy Simpkins, owner of record) – Application for an addition.

Cal #01-23WP

Vitiello residence, 43 Conant Valley Road, Pound Ridge, NY 10576; Sheet 49, Block 9827, Lot 100 (Linda & Michael Vitiello – owners of record) – Application for driveway improvements.

Cal #02-23WP

Ritacco Pool, 37 Gideon Reynolds Road, Cross River, NY 10518; Sheet 16, Block 10533, Lot 423 (George & Eileen Ritacco, owners of record) – Application for a pool.

VII. WETLAND VIOLATION

Cal #01-23WV

VIII. DISCUSSION

<u>Cal #06-17PB</u>

Wolf Conservation Center, Buck Run, South Salem, NY 10590; Sheet 21, Block 10803, Lots 3, 65, 67, 81, 82, 83, 86 & 88 (Wolf Conservation Center, owner of record) - Application for a Subdivision and Special Use Permit associated with a private nature preserve.

IX. SCHEDULE A POST-CONSTRUCTION SITE VISIT

<u>Cal #10-15 PB, Cal #20-17WP, Cal #5-17SW</u> Lewisboro Commons (Wilder Balter), 100 Beekman Lane, Goldens Bridge, NY 10526; Sheet 5, Block 10776, Lots 19, 20 & 21 (Lewisboro Commons Housing Development Fund Co., Inc., owner of record)

- X. MINUTES OF January 17, 2023.
- XI. NEXT MEETING DATE: March 21, 2023.

XII. ADJOURN MEETING.

Ciorsdan Conran

From: Sent: To: Subject: Nancy Tuccillo <nancytuccillo@aol.com> Monday, January 23, 2023 1:33 PM Ciorsdan Conran Re: Permit renewal

Hi Ciorsdan,

We are hereby requesting an extension of the existing permit for the development of the north lot at the North County shopping center in Goldens Bridge.

Kindly let us know if this meets with your approval.

With best regards,

Nancy Tuccillo (914) 769-3141

> On Jan 23, 2023, at 10:34 AM, Ciorsdan Conran < Planning@lewisborogov.onmicrosoft.com> wrote:

>

> Hi Nancy- Know that there has not been an approval for housing in the proposed north building. Please review the attached Planning Board Resolution which excluding the EV chargers is the most recent approval from the Planning Board.

- >
- > Ciorsdan
- >
- > -----Original Message-----
- > From: Nancy Tuccillo <nancytuccillo@aol.com>
- > Sent: Friday, January 20, 2023 2:39 PM
- > To: Ciorsdan Conran <planning@lewisborogov.com>
- > Subject: Permit renewal
- >

> Hi Ciorsdan,

>

> This is to formally request an extension of our building permit for the north lot of the North County Shopping Center in Goldens Bridge. We are proceeding with our original plan, which was to build apartments in that area of the shopping center. We are currently in the middle of doing engineering on the property to determine septic capacity, and plan to proceed thereafter.

>

> If you have any questions, please do not hesitate to call me at 914-769-3141.

Ś

> Very sincerely, yours,

>

- > Nancy Tuccillo
- > (914) 769-3141
- ><Goldens Bridge Village Centre_Request_Extension_granted 021522.pdf>
- > <NorthCountyShoppingCenter_Res stamped 012220.pdf>

This approval is set to expire on 2/17/23

Re: <u>Cal# 4-19PB, #17-19WP, #6-19SW</u> Pound Ridge Stone 2 West Road, South Salem Sheet 49B, Block 9831, Lot 1

From: David Moorman <prstone1@optonline.net> Sent: Wednesday, February 8, 2023 11:07 AM To: Ciorsdan Conran <Planning@lewisborogov.onmicrosoft.com> Subject: Re: Request for extension

To the Planning Board -

Request for a 90-day extension———I have been working on the following paperwork:

- DOT response hasn't been active,
- the wetland mitigation bond is in production, but some questions has arisen and haven't been answered yet,
- the Engineering and Inspection check is ready to be cut,
- mylars are ready,
- I will try to get permit from town stormwater and wetland inspector.

Thank you,

David Moorman Pound Ridge Stone and Landscaping 2 West Rd South Salem NY 10590



MEMORANDUM

TO:	Chairperson Janet Andersen and Members of Lewisboro Planning Board
CC:	Ciorsdan Conran Judson Siebert, Esq. Kevin Kelly, Building Inspector
FROM:	Jan K. Johannessen, AICP Joseph M. Cermele, P.E., CFM Town Consulting Professionals
DATE:	February 24, 2023
RE:	Waccabuc Beach Club Improvements Waccabuc Country Club Perch Bay Road Sheet 25A, Block 10813, Lot 1 Sheet 25, Block 11155, Lot 148

PROJECT DESCRIPTION

The subject property consists of ±9.1 acres of land and is located off of Perch Bay Road within the R-4A Zoning District. The property is owned by the Waccabuc County Club, has frontage on Lake Waccabuc, and is developed with various recreational buildings and amenities to support its private membership. The property obtains access from Perch Bay Road and contains parking areas, a public water well and septic system. The applicant is proposing to renovate the existing boathouse, construct a covered pavilion, replace the existing docks, demolish an existing building, install an outdoor seating area, construct a new snack bar and construct driveways, walkways, handicap access, a new septic system, and stormwater management facilities. The subject property consists of two (2) parcels that are proposed to be merged.

SEQRA

The proposed action has been preliminarily identified as an Unlisted Action pursuant to the State Environmental Quality Review Act (SEQRA). The Planning Board has established itself as the Lead Agency and is conducting a coordinated SEQRA review. Prior to taking action on this pending application, the Planning Board must issue a determination of significance.

CIVIL ENGINEERING | LANDSCAPE ARCHITECTURE | SITE & ENVIRONMENTAL PLANNING

Chairperson Janet Andersen February 24, 2023 Page 2 of 3

REQUIRED APPROVALS/REFERRALS

- 1. Site Development Plan Approval, a Town Stormwater Permit, and a Wetland Activity Permit is required from the Planning Board; a public hearing is underway.
- 2. It is our understanding that the applicant obtained approval and variances from the Zoning Board of Appeals, as required.
- 3. The proposed action was referred to and received approved by the Architecture and Community Appearance Review Council (ACARC).
- 4. A Floodplain Development Permit will be required from the Building Inspector.
- 5. The applicant is proposing new impervious surfaces within the 100-foot limiting distance of the lake and a variance from the New York City Department of Environmental Protection (NYCDEP) is required.
- 6. The proposed septic system will require approval from the Westchester County Department of Health (WCHD). It is noted that the existing well operates as a public water supply.
- 7. The subject property is located within the NYC East of Hudson Watershed and proposed land disturbance equals one (1) acre. Coverage under New York State Department of Environmental Conservation (NYSDEC) SPDES General Permit for Stormwater Discharges from Construction Activity (GP-0-20-001) will be required.
- 8. The application was referred to the Westchester County Planning Board in accordance with Section 239-m of the General Municipal Law, and a letter was received on May 20, 2022.

COMMENTS

- 1. The application has been referred to the Fire Department for review; we await their response.
- 2. Approval of the Stormwater Pollution Prevention Plan (SWPPP) and variance from the NYCDEP is pending.
- 3. The applicant should advise of the status of the WCHD approval for the septic system.

In order to expedite the review of subsequent submissions, the applicant should provide annotated responses to each of the comments outlined herein.

Chairperson Janet Andersen February 24, 2023 Page 3 of 3

PLANS REVIEWED, PREPARED BY INSITE ENGINEERING, DATED FEBRUARY 7, 2023:

- Overall Plan (OP-1)
- Existing Conditions & Removals Plan (EX-1)
- Layout & Landscape Plan (SP-1)
- Grading & Utilities Plan (SP-2)
- Erosion & Sediment Control Plan (SP-3)
- Lighting Plan (LP-1)
- Site Plan (D-1)
- Site Details (D-2, D-3)
- Parking Assessment (PA-1)

DOCUMENTS REVIEWED:

- Letter, prepared by Zach Pearson, P.E., dated February 7, 2023
- Stormwater Pollution Prevention Plan, prepared by Insite Engineering, dated February 7, 2023

JKJ/dc

https://kellardsessionsconsulti.sharepoint.com/sites/Kellard/Municipal/Lewisboro/Correspondence/2023-02-24_LWPB_Waccabuc CC - Perch Bay Road Beach Club_Review Memo.docx

RECEIVED BY

FEB 2 4 2023

Town of Lewisboro

RESOLUTION TOWN OF LEWISBORO ZONING BOARD OF APPEALS IN THE MATTER OF THE APPLICATION OF

Waccabuc Country Club/Beach

ARTICLE III §220-9.E ARTICLE IV §220-23.D.11 ARTICLE IV §220-23.E/Table 220 Attachment 1 ARTICLE IV §220-23.D.11 ARTICLE VII §220-56.D CAL. NO. 02-23-BZ

INTRODUCED BY:	Board Member	Rendo
	board Member	Rendo

SECONDED BY:

.....

Board Member Casper

DATE OF CONSIDERATION/ADOPTION: January 25, 2023

WHEREAS, Michael Fuller Sirignano, Esq., as the applicant (Waccabuc Country Club Co, owner of record) has made application to the Lewisboro Zoning Board of Appeals (the "ZBA"), on the subject premises located at, 18 Perch Bay Road, Waccabuc, NY, Tax Map as Sheet 025A, Block 10813, Lot 001, and (00) Tarry-A-Bit Road, Waccabuc, NY, Tax Map as Sheet 0025, Block 11155 Lot 148, ("the properties"), for the following variances of the Waccabuc Country Club, which is an existing non-conforming use, proposes expansion of the existing non-confirming use which will require review and approval by the ZBA. Per Article III §220-9.E: Extension of Non-Conforming Use of the Town of Lewisboro Zoning Code. The new snack bar proposed is 1,100 square feet of floor area. The Town Code allows a maximum of 600 square feet. Based on this code section, the applicant is requesting a variance of 500 square feet. Per Article IV §220-23.D.11: Accessory Building Floor Area of the Town of Lewisboro Zoning Code. The expansion of the boathouse does not impact current rear yard setback. The existing boathouse projects over the property line 1.4-feet. This condition will remain and require a 50-foot variance from the rear yard setback, plus 1.4-foot projection. The proposed covered pavilion will not increase the current rear yard setback which will remain at 9.1-feet. This will require a 40.9-foot variance from the required rear yard setback. The proposed snack bar will be located 37.4-feet from the rear property line, which is 0.4-feet closer than the existing snack bar. This will require a 12.6-foot variance from the required rear yard setback. Per Article IV §220-23.E/Table 220 Attachment 1: Rear Yard Setback of the Town of Lewisboro Zoning Code. As noted above, the proposed project is an expansion of the current nonconforming use. While additional building area has been added, as well as numerous site features to accommodate accessible access, the use of the property remains the same. Furthermore, the applicants use of 1 parking space for every 3 members of the country club yields 130 required spaces which is more than the calculated or functional occupancy of the property when proposed improvements are complete. While the applicant currently identifies 53 parking spaces onsite, the majority of these parking spaces do not meet the

Waccabuc Country Club / Beach Resolution Cal. No. 02-23-BZ

requirements of the Town Code. Based on the continuation of the sites historical use, we recommend the ZBA consider a variance of 120 spaces. This variance is based on the eight (8) parallel spaces that meet Town Code, as well as the two (2) compliant handicap spaces. *Per Article VII §220-56.D: Parking* of the Town of Lewisboro Zoning Code.

WHEREAS, this application for an area variance constitutes a Type II action under 6 NYCRR Part 617, and therefore, requires no further review under the State Environmental Quality Review Act (SEQRA), and

WHEREAS, a public hearing at the Town Offices, 79 Bouton Road, South Salem, New York in this matter on January 25, 2023, and a site walk was conducted on January 21, 2023 to consider the application, after which a vote was taken with regard to the variances as set forth above, and

WHEREAS, The Lewisboro Zoning Board of Appeals has given careful consideration to the facts presented in the application at the public hearing based upon the criteria set forth in Section 267-b(3)(b) of the Town Law of the State of New York, and finds as follows:

1. The properties are an approximate 6.07-acres parcel and a 3.0-acres parcel in the 4-AC, Four Acre Residential District owned by Waccabuc Country Club Co. The Waccabuc Country Club, which is an existing non-conforming use, and is improved with a recreational beach facility, consisting of a boathouse and snack bar on Lake Waccabuc. The applicant, Waccabuc Country Club Co. wishes to have an expansion of the existing non-confirming use, Per Article III §220-9.E: Extension of Non-Conforming Use of the Town of Lewisboro Zoning Code. The new snack bar as proposed is 1.100 square feet of floor area whereas 600 square feet is allowed Per Article IV §220-23.D.11: Accessory Building Floor Area of the Town of Lewisboro Zoning Code the applicant sought a variance of 500 square feet. The expansion of the boathouse does not impact the current rear yard setback. The existing boathouse projects over the property line 1.4-feet. This condition will remain and require a 50-foot variance from the rear yard setback, plus 1.4-foot projection, the applicant sought a variance of 50' plus a 1.4 'projection from the rear yard setback. The proposed covered pavilion will not increase the current rear yard setback which will remain at 9.1feet. This will require a 40.9-foot variance from the required rear yard setback., the applicant sought a variance of 40.9' from the rear yard setback. The proposed snack bar will be located 37.4-feet from the rear property line, which is 0.4-feet closer than the existing snack bar. This will require a 12.6-foot variance from the required rear yard setback. Per Article IV §220-23. E/Table 220 Attachment 1: Rear Yard Setback of the Town of Lewisboro Zoning Code, the applicant sought a variance of 12.6' from the rear yard setback. As noted above, the proposed project is an expansion of the current non-conforming use. While additional building area has been added, as well as numerous site features to accommodate accessible access, the use of the property remains the same. Furthermore, the applicants' use of 1 parking space for every 3 members of the country club yields 130 required spaces which is more than the calculated or functional occupancy of the property when proposed improvements are

complete. While the applicant currently identifies 53 parking spaces onsite, the majority of these parking spaces do not meet the requirements of the Town Code. Based on the continuation of the site's historical use, we recommend the ZBA consider a variance of 120 spaces. This variance is based on the eight (8) parallel spaces that meet Town Code, as well as the two (2) compliant handicap spaces. The ZBA hereby determines that the minimum area variance necessary in this application is 53 parking spaces, the applicant sought relief of 120 spaces.

- 2. There will be no undesirable change in the character of the neighborhood or detriment to nearby properties.
- 3. There is no practical alternative to the variances requested.
- 4. The Board found that the variances are not substantial.
- 5. There will not be an adverse effect or impact to the physical or environmental conditions of the neighborhood.
- 6. The Board found that the difficulty was partially self-created.

WHEREAS, pursuant to Section 267-b(3)(c), the ZBA hereby determines that the expansion of the existing non-confirming use, is approved Per Article III §220-9.E: Extension of Non-Conforming Use of the Town of Lewisboro Zoning Code.

WHEREAS, pursuant to Section 267-b(3)(c), the ZBA hereby determines that the minimum area variance necessary in this application is 500 square feet from the maximum 600 square feet allowed, Per Article IV §220-23.D.11: of Lewisboro Zoning Code, thereby permitting the construction of the 1.100 square foot snack bar.

WHEREAS, pursuant to Section 267-b(3)(c), the ZBA hereby determines that the minimum variance necessary for the boathouse in this application is 50' plus 1.4-foot projection over the property line/shoreline, from the required 50' rear yard setback, *Per Article IV §220-23.E/Table 220 Attachment 1: Rear Yard Setback* of the Town of Lewisboro Zoning Code, thereby permitting the boathouse to project 1.4' over the rear yard setback.

WHEREAS, pursuant to Section 267-b(3)(c), the ZBA hereby determines that the minimum variance necessary in this application for the covered pavilion is 40.9' from the 50' rear yard setback allowed, *Per Article IV §220-23.E/Table 220 Attachment 1: Rear Yard Setback* of the Town of Lewisboro Zoning Code, thereby permitting the construction of the covered pavilion 9.1' from the rear yard setback.

WHEREAS, pursuant to Section 267-b(3)(c), the ZBA hereby determines that the minimum variance necessary in this application for the proposed snack bar is 12.6' from the rear property line whereas 50' is allowed, *Per Article IV §220-23.E/Table 220 Attachment 1: Rear Yard Setback* of the Town of Lewisboro Zoning Code, thereby permitting the construction of the proposed snack bar 37.4' from the rear yard setback.

WHEREAS, pursuant to Section 267-b(3)(c), the ZBA hereby determines that the 130 required parking spaces is more than the calculated or functional occupancy of the property when

Waccabuc Country Club / Beach Resolution Cal. No. 02-23-BZ

proposed improvements are complete, whereas with 53 parking spaces onsite, with the majority of these parking spaces not meeting the requirements *Per Article VII §220-56.D: Parking* of the Town of Lewisboro Zoning Code, based on the continuation of the site's historical use, thereby permitting a variance of relief of 120 parking spaces, this variance is based on the eight (8) parallel spaces that meet Town Code, as well as the two (2) compliant handicap parking spaces, this is based on the proposed plan as presented.

NOW, THEREFORE BE IT RESOLVED, that the Lewisboro Zoning Board of Appeals hereby grants a variance the expansion of the existing non-confirming use, as approved Per Article III §220-9.E: Extension of Non-Conforming Use of the Town of Lewisboro Zoning Code.

NOW, THEREFORE BE IT RESOLVED, that the Lewisboro Zoning Board of Appeals hereby grants an area variance of 500 square feet from the maximum 600 square feet allowed, thereby permitting the construction of the 1.100 square foot snack bar, Per Article IV §220-23.D.11: of Lewisboro Zoning Code,

NOW, THEREFORE BE IT RESOLVED, that the Lewisboro Zoning Board of Appeals hereby grants a variance of 50' from the rear yard setback plus 1.4' projection over the property line/shoreline, from the required 50' rear yard setback, thereby permitting the boathouse to project 1.4' over the rear yard property line/shore line, *Per Article IV §220-23.E/Table 220 Attachment 1: Rear Yard Setback* of the Town of Lewisboro Zoning Code,

NOW, THEREFORE BE IT RESOLVED, that the Lewisboro Zoning Board of Appeals hereby grants a variance of 40.9' from the allowed 50' rear yard setback, thereby permitting the construction of the covered pavilion 9.1' from the rear yard lot line, *Per Article IV §220-23.E/Table 220 Attachment 1: Rear Yard Setback* of the Town of Lewisboro Zoning Code

NOW, THEREFORE BE IT RESOLVED, that the Lewisboro Zoning Board of Appeals hereby grants a variance of 12.6' from the allowed 50' rear yard lot line, thereby permitting the construction of the snack bar 37.4' from the rear yard lot line, *Per Article IV §220-23.E/Table 220 Attachment 1: Rear Yard Setback* of the Town of Lewisboro Zoning Code

NOW, THEREFORE BE IT RESOLVED, that the Lewisboro Zoning Board of Appeals hereby grants a variance of relief of 120 parking spaces thereby permitting 53 parking spaces, eight (8) parallel parking spaces and two (2) compliant handicap parking spaces as presented at the meeting, *Per Article VII §220-56.D: Parking* of the Town of Lewisboro Zoning Code,

VOTE:

Chair Price	-	In Favor
Board Member Mandelker	-	In Favor
Board Member Casper	-	In Favor
Board Member Infield	-	In Favor
Board Member Rendo	-	In Favor
	-	

VOTE: Resolution carried by a vote of 5 to 0.

Waccabuc Country Club / Beach Resolution Cal. No. 02-23-BZ

Note Mer 2

Robin Price, Jr. Chair Dated in South Salem, New York This 2 / day of February 2023

STATE OF NEW YORK

<u>_</u> *

)) ss.:

COUNTY OF WESTCHESTER

I, Donna Orban, Secretary of the Zoning Board of Appeals, do hereby certify that the above is an excerpt/summary/fair representation of the Resolution adopted by the Zoning Board of Appeals of the Town of Lewisboro at a meeting of said Board on January 25, 2022.

Dated: February 24 2033

sten Donna Orban

Secretary Zoning Board of Appeals

January 24, 2023



Rohit T. Aggarwala Commissioner

Paul V. Rush, P.E. Deputy Commissioner Bureau of Water Supply prush@dep.nyc.gov

465 Columbus Avenue Valhalla, NY 10595 T: (845) 340-7800 F: (845) 334-7175 Mr. Zac Pearson, P.E. Sr. Associate/ Project Engineer Insite Engineering, P.C. 3 Garrett Place Carmel, New York 10512

Via email: zpearson@insite-eng.com

Re: Waccabuc Country Club-Beach Club Improvements - Variance Perch Bay Road Town of Lewisboro Tax Map ID: 32.4-3-13&14 Cross River Reservoir Drainage Basin DEP Log # 2022-CR-0297-VA.1

Dear Mr. Pearson:

The New York City Department of Environmental Protection (DEP) has reviewed the last submission regarding the above referenced project. Please be advised that the following comments should be addressed satisfactorily in order to provide a final determination

- 1. Based on the proposed design, the new and redeveloped impervious areas from Drainage Area 1.1S is routed to the proposed P-1 Pond. The starting elevation per the Hydrocad model is at 480 ft and the groundwater elevation per the soil testing observed was at 485. As such, it appears that the pond would be continuously bleeding out and there won't be any storage available for the incoming flow. Therefore, demonstrate how the requirement for 24 hours extended detention of the Channel Protection Volume (CP) is achieved as mentioned in the report and in the previous response letter. Please run the model for more than 24hours by setting the permanent pool elevation at or above the groundwater elevation and note that storage cannot be assumed below the groundwater elevation. Please revise.
- 2. Response #8 from your last submission must be corrected in the variance report based on how the above comment is addressed.
- 3. Provide backup from Hydrocad to demonstrate that how the higher storms (10 & 100-yr) are attenuated with the existing high groundwater condition at the proposed pond(P-1) location.
- 4. The inflow pipe to the pond must be extended to reach the actual forebay. The inlet point to the forebay should be stabilized to ensure non-erosive conditions

exist for at least the 2-year storm event. Also, a non-erosive rip rap channel must be provided to the micropool from the forebay.

- 5. The outlet from the pond should be connected to the micropool and not to the forebay to avoid short-circuiting. Also, see that the maintenance access extends to the micropool for ease of maintenance as shown in the Design Manual.
- 6. Include the pond drain elevation(C) for the micro pool detention pond on the plans. Using a sump pump to fully dewater the pond in the event of future maintenance shall be noted on the pond detail.
- 7. Locate level spreader B on the plan. No level spreader is shown at ES9B, inflow point to the P-1 Pond. Explain why a level spreader is chosen at the inflow point instead of a Rock Outlet Protection(ROP). Also, an ROP for the cistern overflow shall be included on the plans as requested in the previous comment letter.
- 8. A significant amount of flow enters the level spreader A at ES2 from sides and therefore a uniform flow distribution will not be possible as mentioned in the Blue Book. Revise the orientation of the pipe outlet so that these lateral flows are avoided.
- 9. Provide the velocity of the flow at ES9A in the 100 yr storm. This is not provided in the Hydrocad and as such requested for in the pipe sizing calculations in the last comment letter but nor provided.
- 10. Provide a call-out for the aquatic bench on the enlarged pond plan. The aquatic bench as it appears on the landscape plan should be located around the forebay and micropool separately as shown in the Design Manual. Indicate whether these plants are suitable enough to withstand the groundwater conditions in the proposed area of the pond.
- 11. Based on the detail provided for the temporary sediment trap and the groundwater elevation shown on the profile, explain what storage will be available in order to use it as temporary erosion control practice.
- 12. Identify the specific 13 trees utilized for the reduction of impervious surfaces drainage area 1.2S.
- 13. The WQv and the RRV shown in the RRV calculation in Appendix A for the cistern does not match with what is provided in the Hydrocad. The numbers should be consistent with Table 2.2.1.

- 14. Provide a reference for the recommend water demand per plant as used in Table 2.2.2. In addition, the area to be irrigated per Table 2.2.2. must be called out on the plan.
- 15. Demonstrate that the pump provided on the plans in relation to cistern is sized enough to meet the irrigation needs.

If you have any questions, I may be reached at (914)749-5357 or <u>mzachariah@dep.nyc.gov</u>.

Sincerely,

Mariyam Zachariah

Mariyam Zachariah Associate Project Manager EOH Project Review Group Regulatory and Engineering Programs

C: Peter Hall, Club Member, <u>peterjhall@gmail.com</u> John Assumma, General Manager, <u>jdassumma@optonline.net</u> Town of Lewisboro Planning Board - <u>planning@lewisborogov.com</u> Jan Johannessen, Town Planner, <u>jjohannessen@kelses.com</u> Matthew Giannetta, CPSWQ, DEP - <u>mgiannetta@dep.nyc.gov</u> Danny Shedlo, P.E. DEP - <u>DShedlo@dep.nyc.gov</u>



February 7, 2023

Town of Lewisboro Planning Board Janet Andersen, Planning Board Chair 79 Bouton Road South Salem, NY 10590

RE: Beach Club Improvements Project Waccabuc Country Club Perch Bay Road Waccabuc, NY TM# Block 11155, Lot 148, Sheet 25 and Block 10813, Lot 01, Sheet 25A

Chair Anderson and Members of the Board:

Please find enclosed the following plans provided in support of an application for site development plan, wetland permit and stormwater permit approvals for the above referenced project:

- Site Plan Set (9 Sheets Total), last revised February 7, 2023.
- Stormwater Pollution Prevention Plan, dated February 7, 2023.
- Figure PA-1 Parking Assessment (formerly PS-1 Parking Study), last revised February 7, 2023.

Since we last met with the Board on October 18, 2022, we received a denial / referral letter from the Building Inspector and have met with the Zoning Board of Appeals at their January 25, 2023 meeting. The ZBA granted the necessary setback variances for the Boathouse, the Pavillion and the floor area and setback variances for the Snackbar. In addition, the ZBA approved the Special Permit for the Beach Club for the current project, and a parking variance to permit up to 53 cars to park at the site.

Drawing OP-1 has been updated to note the variances granted by the ZBA and Figure PA-1 has been updated to note the variance granted by the ZBA for the parking at the site.

Since our last appearance before the board, the project has been submitted to the Westchester County Department of Health (WCDOH) for their review of the onsite wastewater treatment system (OWTS) and the New York City Department of Environmental Protection (NYCDEP) for the impervious surface variance. We have recently received comments from the NYCDEP on the technical aspects of variance and the enclosed plans are inclusive of our most recent responses to the outstanding items.

We look forward to meeting with the Board at the February 28, 2023 meeting. At that time, we will respectfully request that the Board consider closing the public hearing and voting on conditional site plan approval for the project.

Should you have any questions or comments regarding this information, please feel free to contact our office.

Very truly yours,

INSITE ENGINEERING, SURVEYING & LANDSCAPE ARCHITECTURE, P.C.

Page 2 of 2

February 7, 2023

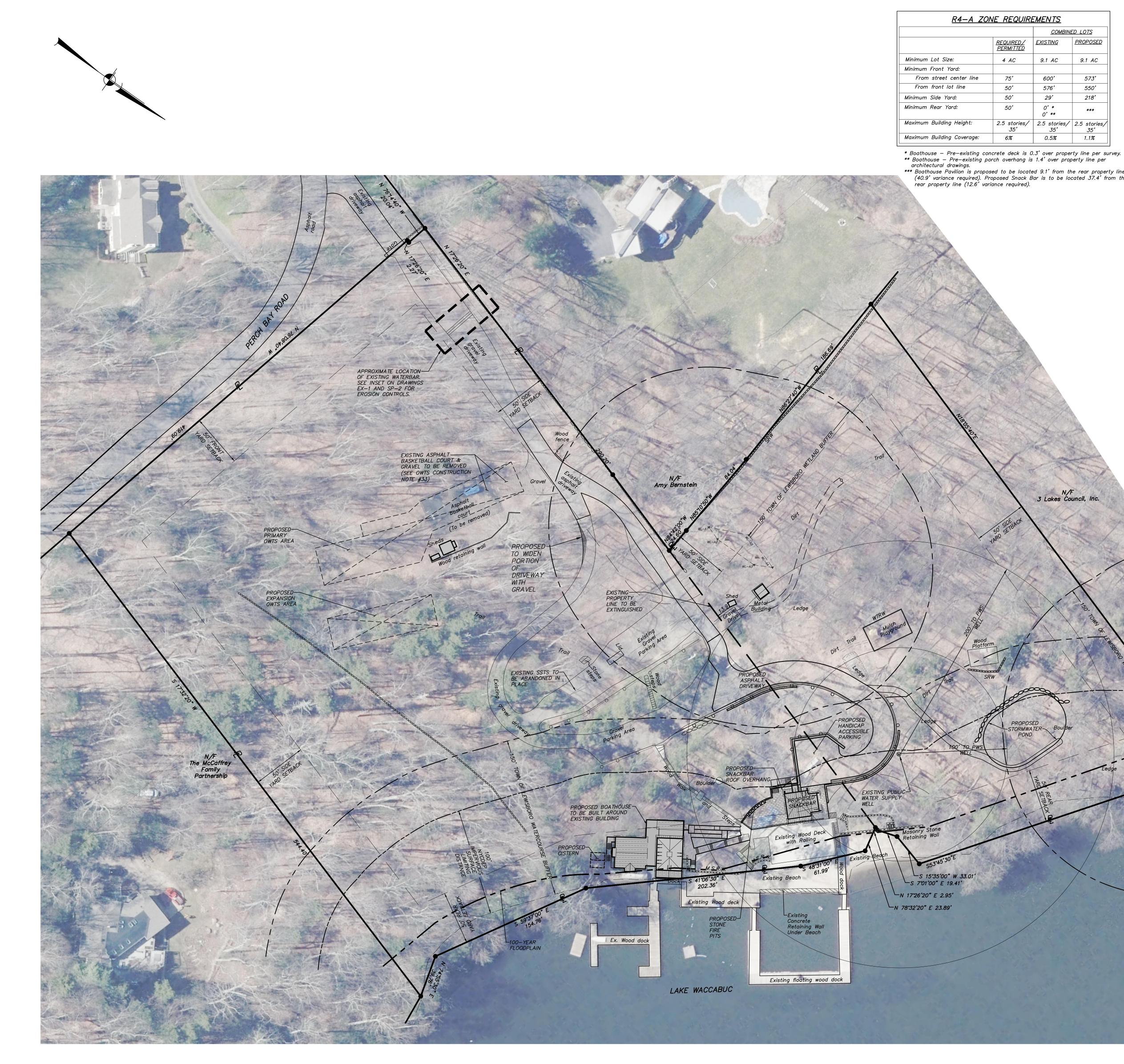
By: Zachary M. Pearson, PE, Sr. Associate Senior Project Engineer

ZMP/dlm

Enclosures

- cc: Mr. Peter Hall, via email
 - Mr. John Assumma, via email
 - Mr. Michael Sirignano, Esq., via email
 - Mr. John M. Doyle, AIA, Doyle Coffin Architecture LLC, via email
 - Mr. Jan Johannessen, via email

Insite File No. 20228.100

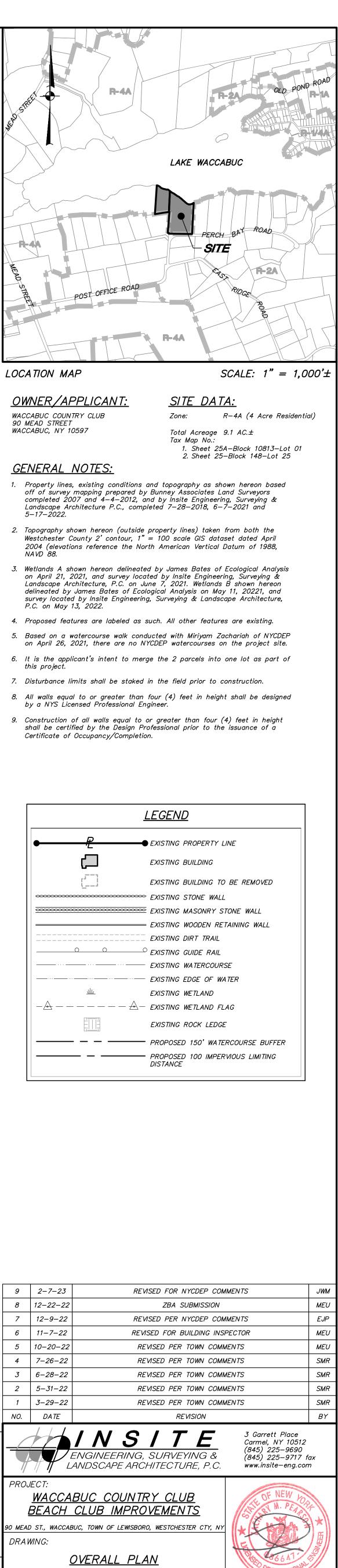


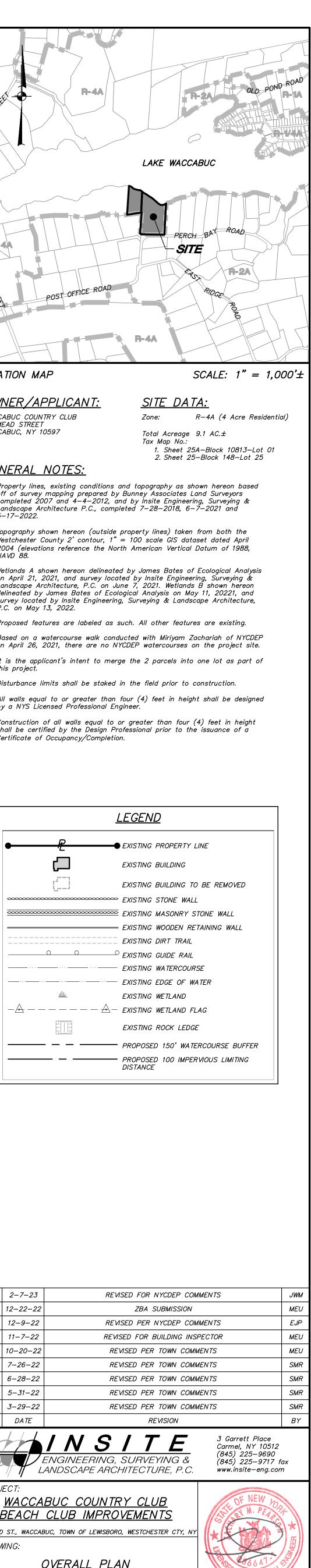
	Approved by
GRAPHIC SCALE	 Chair
(IN FEET) 1 inch = 40 ft.	Administrato

REQUIREMENTS			
	<u>COMBINED LOTS</u>		
REQUIRED/ PERMITTED	<u>EXISTING</u>	<u>PROPOSED</u>	
4 AC	9.1 AC	9.1 AC	
75'	600'	573 '	
50'	576'	550'	
50'	29'	218'	
50'	0' * 0' **	***	
2.5 stories/ 35'	2.5 stories/ 35'	2.5 stories/ 35'	
6%	0.5%	1.1%	

PARKING TABLE <u>Parking Requirements:</u> For Golf or Country Club per Section 220–56 of Town of Lewisboro Zoning Code = 130 spaces 1 space per 3 members x 390 members (*) 1 space per each 3 seats of max. capacity (**) = 0 spaces Total spaces required = 130 spaces (*) Waccabuc Country Club current membership is 390 members. (**) Meeting and dining room seats provided at main club site, not at Beach Club. Parking Provided on site ****: = 53 spaces (***) (***) Parking variance granted for 77 spaces by Town of Lewisboro ZBA on 1–25–2023. (****) Existing on site parking has historically proven sufficient for typical uses at the site. When additional event parking is needed, it is provided at the main club and members/guests are shuttled to/from the site via club vans. Parking provided on site is seasonal. The parking

areas are gravel and partially grass and pavement markings are not provided. This drawing provides a general representation of parking at site.





		9	2-7-23		
		8	12-22-22		_
	-	7	12-9-22		
		6	11-7-22		_
		5	10-20-22		_
		4	7–26–22		_
		3	6–28–22		
		2	5-31-22		
-		1	3–29–22		_
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he undersigned is the owner(s) of the property shown hereon, is familiar with ts contents, and hereby approves the same for filing.		PROJE NUMBE		228.100	
Vaccabuc Country Club Date	·	DATE	2-	8–22	

SCALE 1" = 40'

** Boathouse – Pre-existing porch overhang is 1.4' over property line per architectural drawings. *** Boathouse Pavilion is proposed to be located 9.1' from the rear property line (40.9' variance required). Proposed Snack Bar is to be located 37.4' from the rear property line (12.6' variance required).

> 3 Lakes Council, Inc. 5 82°05'00" E

PLANNING BOARD APPROVAL

by the Resolution of the Lewisboro Planning Board.

Date

Date

Waccabuc Country Club 90 Mead Street Waccabuc, NY 10597

PROJECT MANAGER SHEET DRAWING NO. Z.M.P. DRAWN BY OP - i*E.R.A*. CHECKED BY D.L.M.



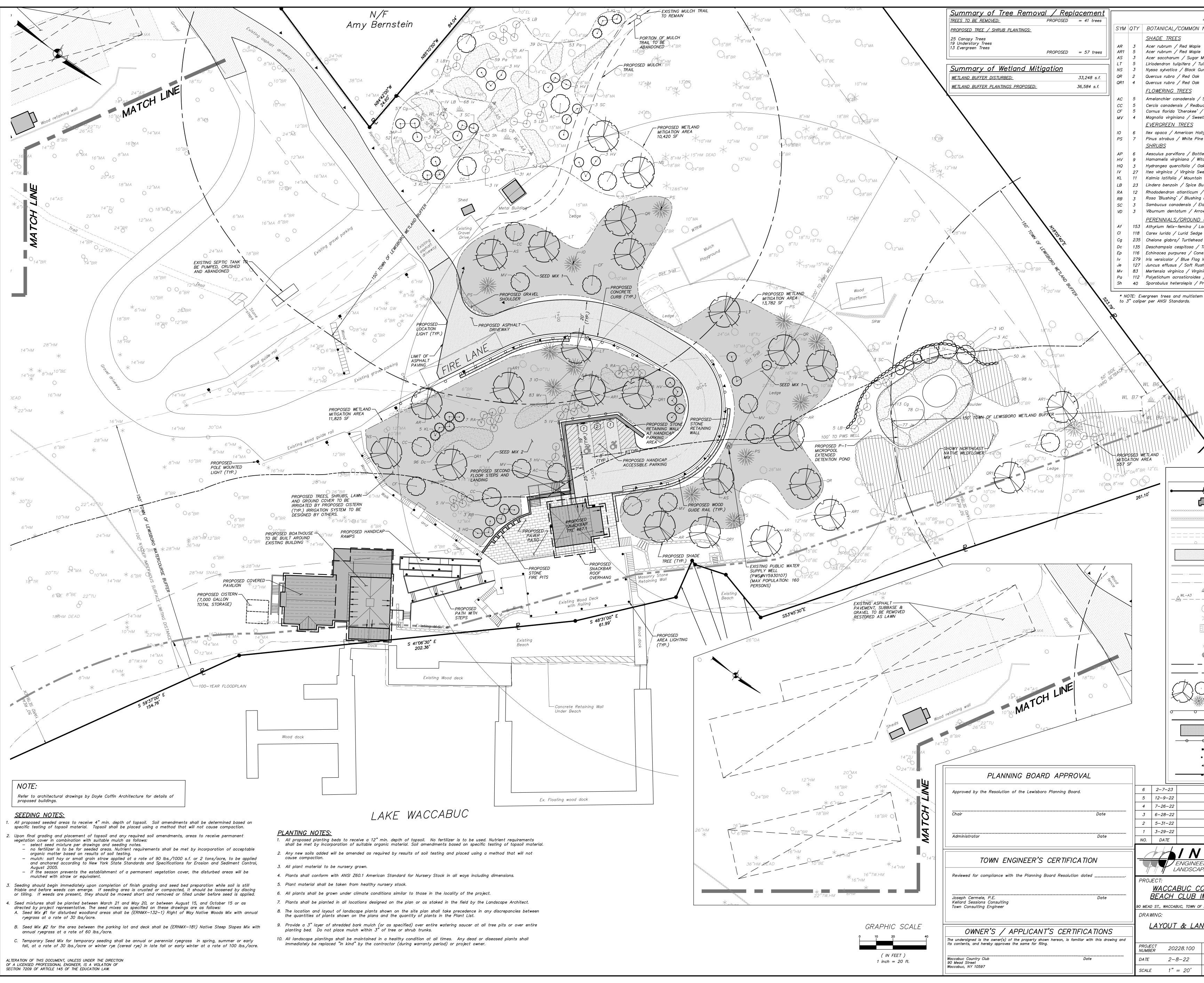
	<u></u>	<u>REE REMOVAL CI</u>	<u>HART</u>
CONDITION	TREE NUMBER	TREE SPECIES	CONDITION
GOOD	29	15" & 6" MAPLE	GOOD
FAIR	30	8" MAPLE	GOOD
GOOD	31	12" MAPLE	GOOD
FAIR	32	20" TULIP	GOOD
FAIR	33	18" TULIP	FAIR
FAIR	34	8" BIRCH	GOOD
GOOD	35	18" TULIP	POOR
FAIR	36	10" & 16" MAPLE	POOR
GOOD	37	12" MAPLE	POOR
GOOD	38	24" TULIP	FAIR
GOOD	39	22" TULIP	FAIR
GOOD	40	8" BIRCH	POOR
GOOD	43	10" BIRCH	GOOD

TREE NUMBER	TREE SPECIES	CONDITION
44	8" BIRCH	GOOD
45	10" MAPLE	GOOD
46	10" MAPLE	GOOD
47	10" MAPLE	GOOD
48	8" MAPLE	GOOD
49	8" MAPLE	GOOD
50	10" BLACK BIRCH	GOOD
51	8" BIRCH	GOOD
52	10" MAPLE	FAIR
53	20" TULIP	POOR
54	20" TULIP	POOR
55	12" BIRCH	FAIR
56	16" BEECH	POOR
57	10" MAPLE	POOR
Trees to be	Removed:	40 (611" Cal.)

EXISTIN	G PROPERTY LINE
EXISTIN	G BUILDING
EXISTIN	G BUILDING TO BE REMOVED
EXISTIN	G STONE WALL
EXISTIN	G MASONRY STONE WALL
EXISTIN	G WOODEN RETAINING WALL
EXISTIN	G DIRT TRAIL
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EXISTIN	G SPOT GRADE
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EXISTIN	G EVERGREEN TREE
EXISTIN	G TREE TO BE REMOVED
EXISTIN	G ROCK LEDGE
EXISTIN	G POST MOUNTED LIGHT



REVISED FOR NYCDEP COMMENTS	JWM
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S / T E ERING, SURVEYING & PE ARCHITECTURE, P.C. 3 Garrett Place Carmel, NY 10512 (845) 225–9690 (845) 225–9717 fo www.insite-eng.con	
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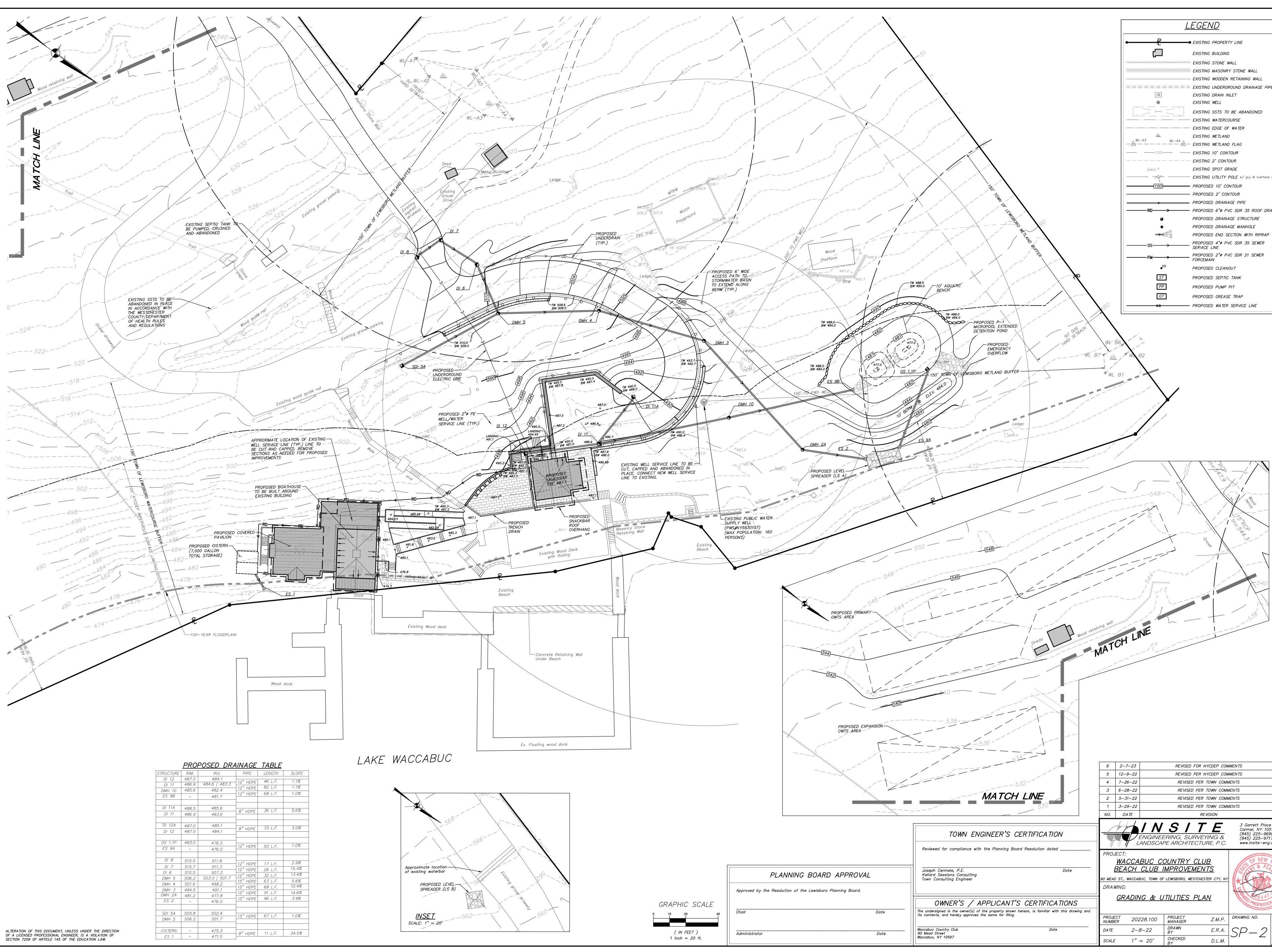


<u>PLANT LIST</u>		
NAME	SIZE	ROOT/NOTES
	3" CAL.	B&B
	2" CAL.	B&B
Maple	3" CAL.	B&B
ulip Tree	3" CAL.	B&B
um	3" CAL.	B&B
	3" CAL.	B&B
	2" CAL.	B&B
Serviceberry	6' Ht.	B&B/(Multi-stem)
ud	3" CAL.	B&B
/ Flowering Dogwood	3" CAL.	B&B
etbay Magnolia	10' HT.*	B&B/(Multi–stem)
lly	10' HT.*	B&B
e	10' HT.*	B&B
-		
lebrush Buckeye	3' HT.	#3 CONT./6'O.C.
itchhazel	3' HT.	#3 CONT./6' O.C.
akleaf Hydrangea	3' HT.	#3 CONT./6' O.C.
veetspire	2' HT.	#3 CONT./6' O.C.
n Laurel	3' HT.	#3 CONT./6' O.C.
lush	2' HT.	#3 CONT./6' O.C.
/ Deciduous Azalea	3' HT.	#3 CONT./6' O.C.
Knockout Rose	2' HT.	#3 CONT./6' O.C.
Iderberry	2' HT.	#3 CONT./6' O.C.
owwood Viburnum	2' HT.	#3 CONT./6' O.C.
<u>COVERS</u>		11 CONT 48" 0 0
ady Fern		#1 CONT./18" O.C.
e d		2" PLUG/18" O.C.
d Trill III i O		2" PLUG/18" O.C.
Tufted Hair Grass		#1 CONT./18" O.C.
eflower Iric		#1 CONT./18" O.C.
lris		2" PLUG/18" O.C.
sh nia Bluebells		2" PLUG/18" O.C. #1 CONT./18" O.C.
/ Christmas Fern		#1 CONT./18" O.C.
Prairie Dropseed		#1 CONT./18" O.C.
,		"······///////////////////////////////

* NOTE: Evergreen trees and multistem trees are sold by height. 10' height is equivalent

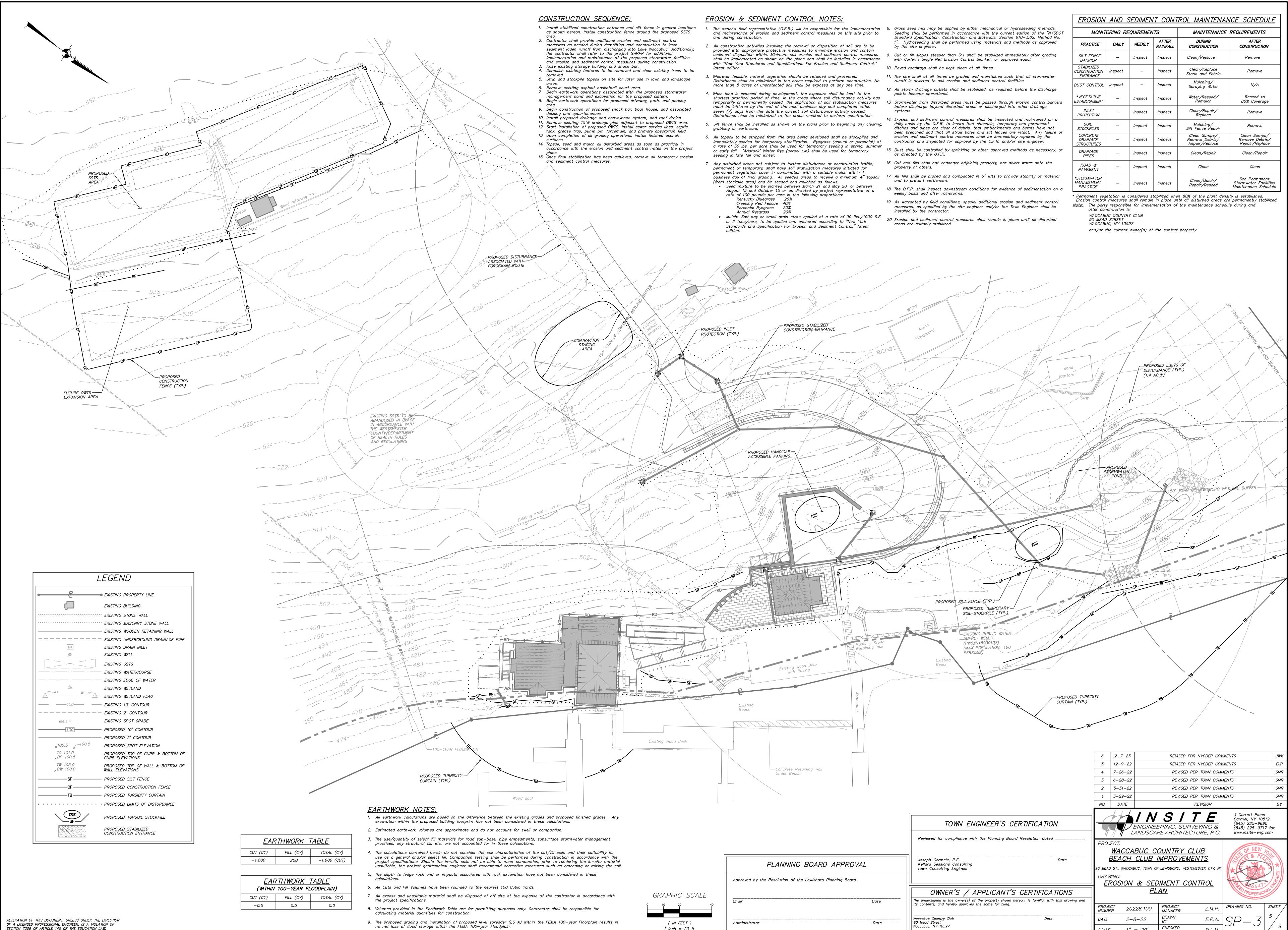
<u></u>	<u>PEE LEGEND</u>
\bigcirc	DECIDUOUS TREE
×	EVERGREEN TREE
AS	ASH
BB	BLACK BIRCH
BE	BEECH
BR	BIRCH
НК	HICKORY
НМ	HEMLOCK
МА	MAPLE
OA	OAK
TR	UNKNOWN SPECIES
ΤU	TULIP

<u></u>	<u>EGEND</u>		
<u>₽</u>	EXISTING PRO	PERTY LINE	
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	EXISTING WATE	ERCOURSE	
	EXISTING EDGE		
<u></u>	EXISTING WETL	AND	
<u>WL-A4</u>			
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	EXISTING POS	T MOUNTED LIGHT	
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WL-A4	— EXISTING WETLAND FLAG
	- EXISTING 10' CONTOUR
	- EXISTING 2' CONTOUR
	EXISTING SPOT GRADE
	— EXISTING UTILITY POLE w/ guy & overhead wires
	- PROPOSED 10' CONTOUR
	- PROPOSED 2' CONTOUR
	PROPOSED DRAINAGE PIPE
	– PROPOSED 6"ø PVC SDR 35 ROOF DRAIN
	PROPOSED DRAINAGE STRUCTURE
ন্দ	PROPOSED DRAINAGE MANHOLE
	PROPOSED END SECTION WITH RIPRAP
	_ PROPOSED 4"ø PVC SDR 35 SEWER SERVICE LINE
	_ PROPOSED 2"ø PVC SDR 21 SEWER FORCEMAIN
	PROPOSED CLEANOUT
	PROPOSED SEPTIC TANK
	PROPOSED PUMP PIT
	PROPOSED GREASE TRAP
	- PROPOSED WATER SERVICE LINE

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no net loss of flood storage within the FEMA 100-year Floodplain.

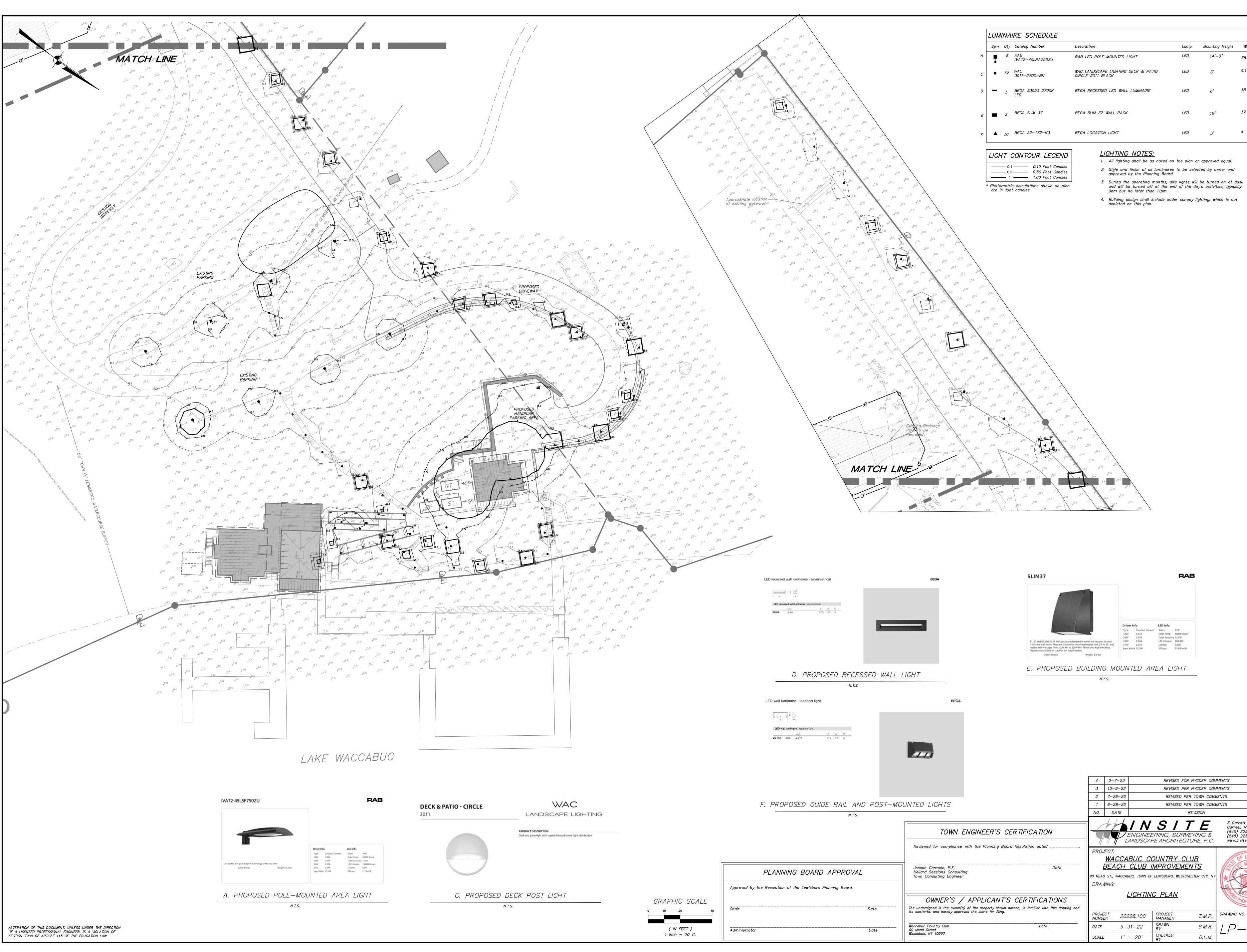
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*VEGETATIVE ESTABLISHMENT	_	Inspect	In
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SOIL STOCKPILES	_	Inspect	In
CONCRETE DRAINAGE STRUCTURES	_	Inspect	In
DRAINAGE PIPES	_	Inspect	In
ROAD & PAVEMENT	_	Inspect	In
*STORMWATER MANAGEMENT PRACTICE	_	Inspect	In

1" = 20'

SCALE

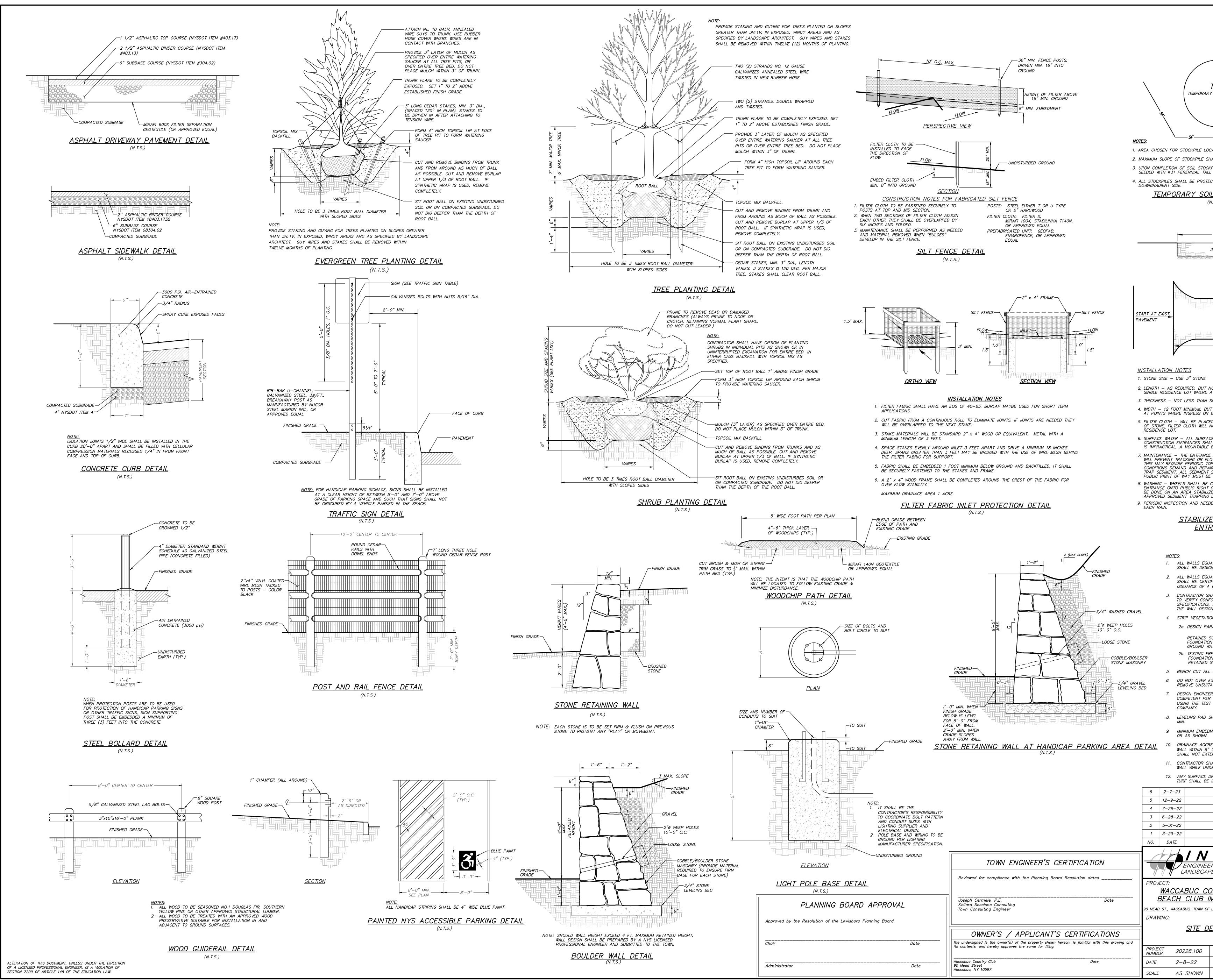
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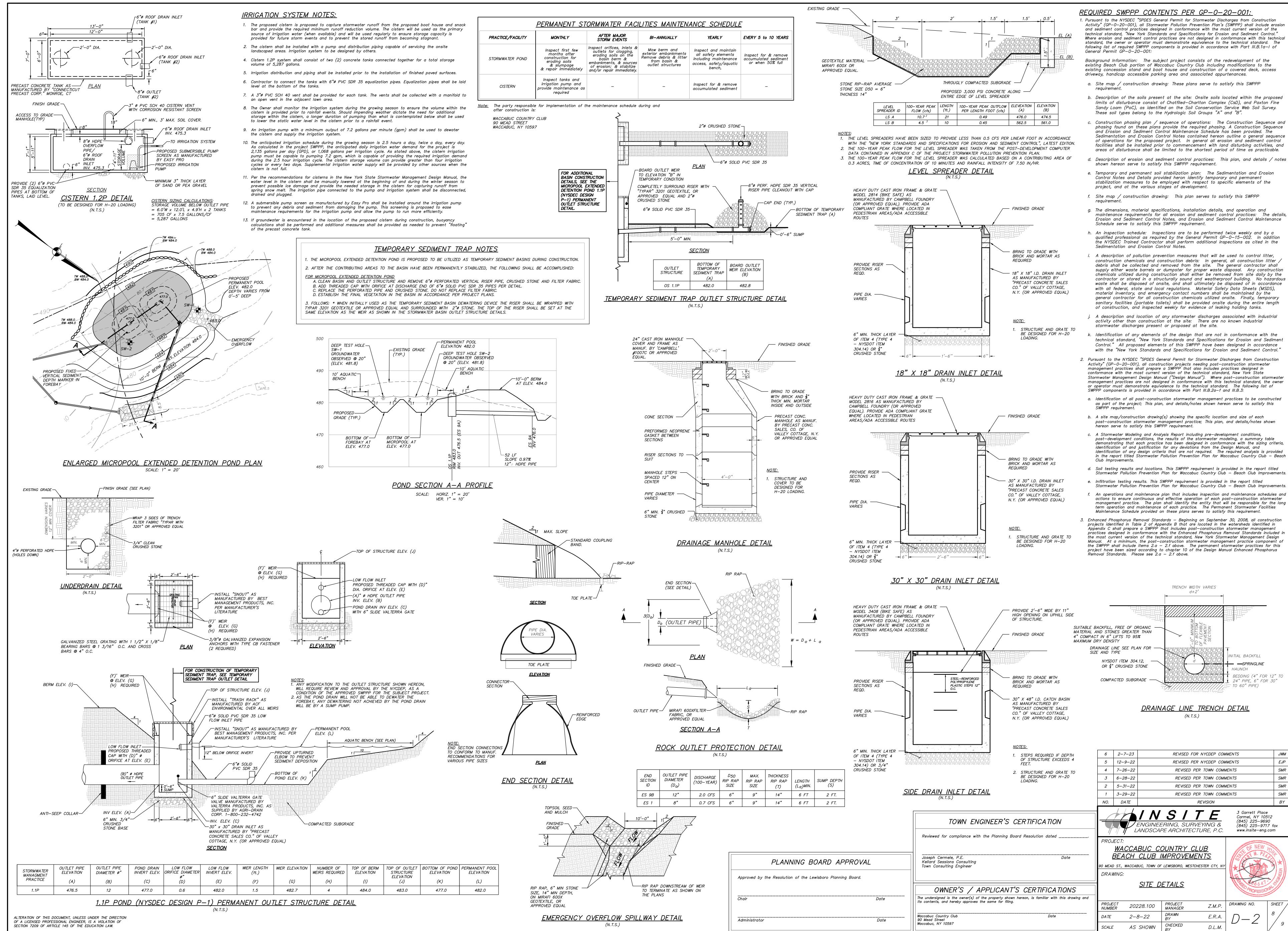
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PLANNING BOARD APPROVAL	Joseph Cermele, P.E. Date Kellard Sessions Consulting Town Consulting Engineer		ACCABUC, TOWN
the Resolution of the Lewisboro Planning Board.		DRAWING:	
	OWNER'S / APPLICANT'S CERTIFICATIONS		<u>LIGHTII</u>
Date	The undersigned is the owner(s) of the property shown hereon, is familiar with this drawing and its contents, and hereby approves the same for filing.	PROJECT NUMBER	20228.100
	Waccabuc Country Club Date	DATE	5-31-22

	Lamp	Mounting Height	Watts
	LED	14'-0"	38
ΠΟ	LED	3'	5.1
	LED	6'	38.4
	LED	18'	37
	LED	3'	4

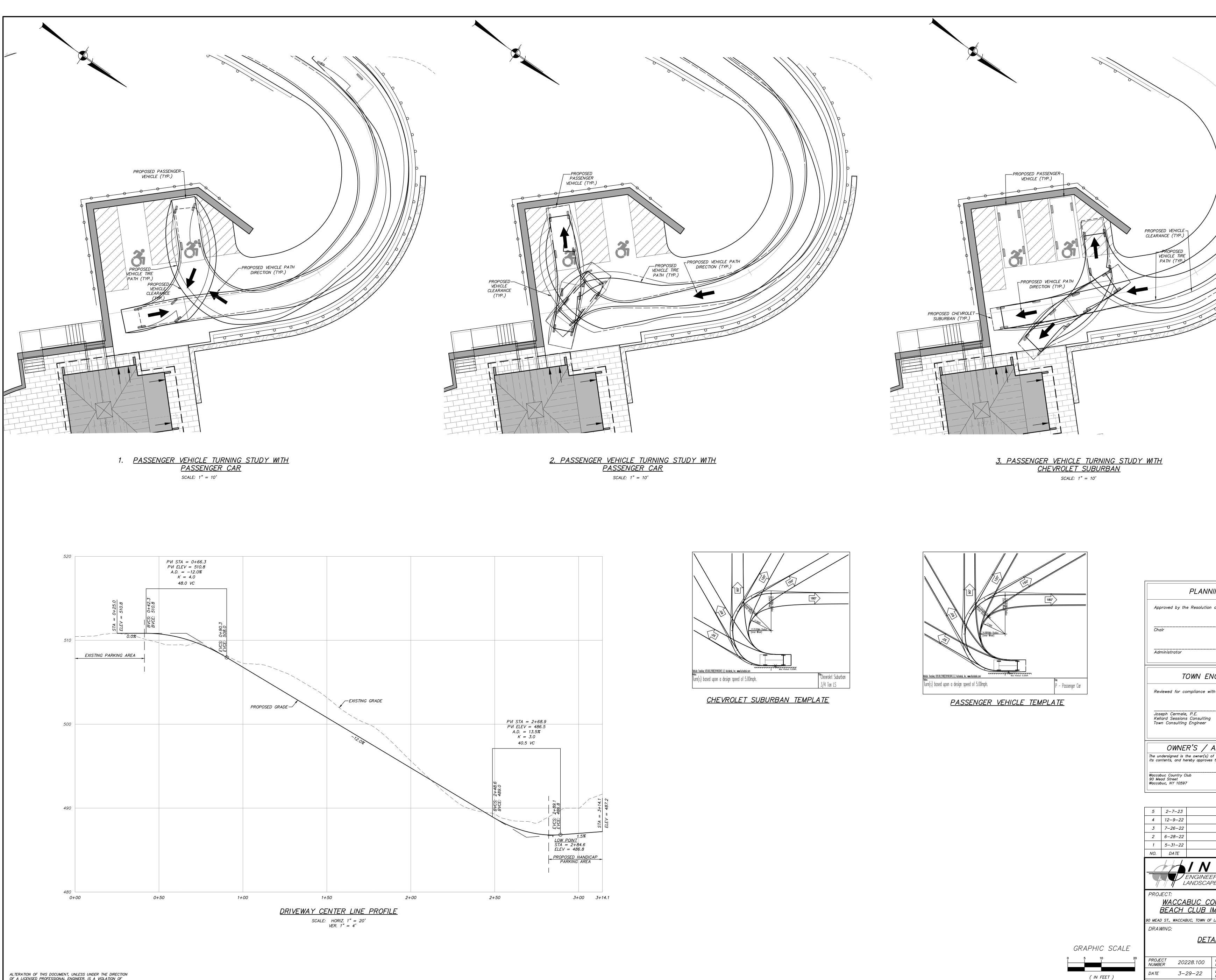
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I <u>L_STOCKPILE</u> ^(N. T. S.)	<u>DE 1 A</u>	<u>\//_</u>	
-12' MIN. WIDTH		6" MIN.	
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	_	:-                -    -	
——— 50' MIN. LENGTH [.] <u>PLAN</u>			
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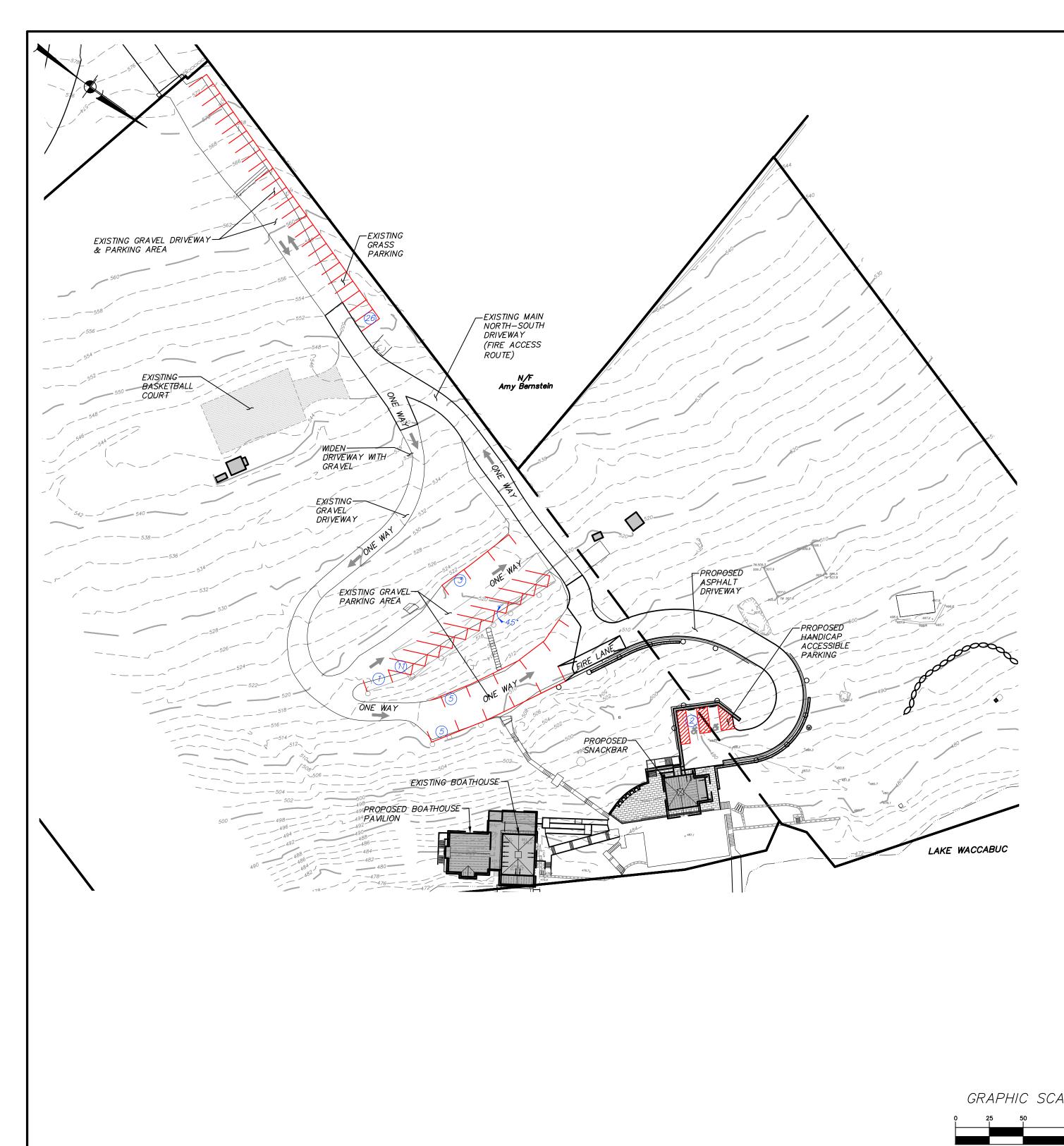
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ALTERATION OF THIS DOCUMENT, UNLESS UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER, IS A VIOLATION OF SECTION 7209 OF ARTICLE 145 OF THE EDUCATION LAW.

PROPOSED VEHICLE CLEARANCE (TYP.) PROPOSED VEHICLE TIRE PATH (TYP.)
WITH
PLANNING BOARD APPROVAL
Approved by the Resolution of the Lewisboro Planning Board.         Chair       Date         Administrator       Date         Image: Town Engineer's Certification         Reviewed for compliance with the Planning Board Resolution dated
Kellard Sessions Consulting Town Consulting Engineer         OWNER'S / APPLICANT'S CERTIFICATIONS         The undersigned is the owner(s) of the property shown hereon, is familiar with this drawing and its contents, and hereby approves the same for filing.         Waccabuc Country Club 90 Mead Street Waccabuc, NY 10597
5       2-7-23       REVISED FOR NYCDEP COMMENTS       JWM         4       12-9-22       REVISED PER NYCDEP COMMENTS       EJP         3       7-26-22       REVISED PER TOWN COMMENTS       SMR         2       6-28-22       REVISED PER TOWN COMMENTS       SMR         1       5-31-22       REVISED PER TOWN COMMENTS       SMR         NO.       DATE       REVISED PER TOWN COMMENTS       SMR         NO.       DATE       REVISED PER TOWN COMMENTS       BY         Image: Comparison of the text of the text of t
WACCABUC COUNTRY CLUB BEACH CLUB IMPROVEMENTS90 MEAD ST., WACCABUC, TOWN OF LEWISBORO, WESTCHESTER CTY, NYDRAWING:DRAWING:PROJECT NUMBER20228.100PROJECT NUMBER20228.100PRAWN BYSCALEAS SHOWNCHECKED BYD.L.M.

1 inch = 10 ft.



oro Zoning Code
= 130 spaces
= 0 spaces
= 130 spaces
not at Beach Club.
= 53 spaces (***)

(***) Parking variance granted for 77 spaces by Town of Lewisboro ZBA on 1—25—2023.

(****) Existing on site parking has historically proven sufficient for typical uses at the site. When additional event parking is needed, it is provided at the main club and members/guests are shuttled to/from the site via club vans. Parking provided on site is seasonal. The parking areas are gravel and partially grass and pavement markings are not provided. This drawing provides a general representation of parking at site.

	5 2–7–23 PLANNING BOARD SUBMISSION					MEU	
	4 11–7–22 REVISED FOR BUILDING INSPECTOR					SMR	
CABUC	3 10–20–22 REVISED PER TOWN COMMENTS					MEU	
	2	9–27–22		REVISED F	PER TOWN COMM	IENTS	MEU
	1 5–31–22 REVISED PER TOWN COMMENTS						SMR
	NO.	DATE			REVISION		BY
APPHIC SCALE							fax
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( IN FEET )	DATE	3-2	29–22	DRAWN BY	S.M.R.	PA-1	1
1 inch = 50 ft.	SCALE	· 1" ·	= 50'	CHECKED BY	D.L.M.		/ 1



# STORMWATER POLLUTION PREVENTION PLAN

For

Waccabuc Country Club – Beach Club Improvements Town of Lewisboro, New York

February 7, 2023

Applicant Information:

Waccabuc Country Club 90 Mead Street Waccabuc, New York 10597

Note: This report in conjunction with the project plans make up the complete Stormwater Pollution Prevention Plan.

Prepared by: Insite Engineering, Surveying & Landscape Architecture, P.C. 3 Garrett Place Carmel, New York 10512

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#### **APPENDICES**

Appendix A	RRv Calculations
Appendix B	Pre-Development Computer Data
Appendix C	Post-Development Computer Data
Appendix D	NYSDEC SPDES for Construction Activities Construction Site Log Book
Appendix E	Project and Owner Information
Appendix F	NYSDEC Stormwater/Wetland Pond Construction and Maintenance Checklist
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Appendix H	Draft NYSDEC Notice of Intent and MS4 SWPPP Acceptance Form
Appendix I	Temporary Sediment Trap Sizing Calculations
Appendix J	Draft Stormwater Maintenance Agreement
Appendix K	Rock Outlet Protection Sizing

# FIGURES

- Figure 1: Location Map
- Figure 2: Pre-Development Drainage Map Figure 3: Post-Development Drainage Map
- Figure 4: Impervious Redevelopment Plan
- Figure 5: Testing Plan

#### 1.0 INTRODUCTION

#### 1.1 Project Description

The subject project is located on two (2) parcels totaling  $9.1 \pm acres$  on Perch Bay Road in the Town of Lewisboro. The parcels and their surroundings are delineated on the attached Location Map (Figure 1). The parcels are designated as Tax Map Number 32.4-3-13 and 32.4-3-14 and are located in the R-4A zoning district. The subject parcels are currently developed as the Beach Club portion of Waccabuc Country Club with a concession stand, boat house, storage building with associated decking, parking areas and appurtenances. The project is located in the Amawalk Watershed.

The subject project consists of the redevelopment of existing impervious areas and new impervious surfaces. The existing concession stand and boat house are proposed to be modified. The project also proposes to construct a covered deck, access driveway, handicap accessible parking area and associated appurtenances. The proposed new and redeveloped impervious areas will be treated in a stormwater pond and a cistern.

#### 1.2 Existing Site Conditions (Pre-Development)

The subject property consists of two adjoining parcels located on the north side of Perch Bay Road in the Town of Lewisboro. The subject parcels are currently developed as a the Beach Club portion of Waccabuc Country Club with a concession stand, boat house, storage building with associated decking, parking areas and appurtenenaces. The existing ground cover on the site is characterized as mostly wooded with lawn and impervious surfaces. The parcels front Lake Waccabuc to the north. The site generally drains from south to north towards Lake Waccabuc.

This analysis included in the project SWPPP utilized one Design Line (Design Line 1) to assess the stormwater runoff and any potential impacts from the proposed development to the existing downstream natural resource, Lake Waccabuc. Design Line 1 is located along a portion of the shoreline of Lake Waccabuc on the north side of the project site. The Pre-Development Drainage Map (Figure 2) shows the location of Design Line 1. The contributing area to Design Line 1 is identified as subcatchment Pre 1.

The hydrologic soils groups for the project consists of "B" and "C". The designations of the onsite soils located within the proposed limits of disturbance consist of Chatfield-Charlton Complex (CsD), and Paxton Fine Sandy Loam (PoC) as identified on the Soil Conservation Service Web Soil Survey. The soils boundaries are shown on Figure 2 and 3 of this report.

#### 1.3 Proposed Site Conditions (Post Development)

As previously stated, the project includes the construction of a new impervious areas and redevelopment of existing impervious areas. The project proposed to modify the existing concession stand and boat house as well as construct a covered deck, access driveway, handicap accessible parking area and associated appurtenances. The proposed new and redeveloped impervious areas across the project site will be treated in a stormwater pond and a cistern. The cistern will be used to irrigate the proposed landscaping in the area of the proposed improvements.

The areas of redevelopment will be treated in accordance with Chapter 9, Redevelopment, of the New York State Stormwater Management Design Manual (Design Manual).

Treatment of stormwater runoff for all newly created impervious surfaces will meet the sizing requirements of Chapter 4 and 10 of the Design Manual. This will result in new SMP's designed to capture and treat runoff from the impervious surfaces. Stormwater treatment for the new impervious surfaces will be accomplished with several different practices including a P-1 Micropool Extended Detention Pond and a cistern. The pond and cistern have been sized to capture and treat the Runoff Reduction Volume and Water Quality Volume from the new and redeveloped areas.

1

The developed / redeveloped subcatchments in the post-development conditions tributary to the Design Line that require treatment are shown as subcatchment 1.1S and 1.2S in Figure 3 of this report. The untreated / undeveloped portion of the contributing area to the Design Line is shown as subcatchment 1.0S.

#### 2.0 STORMWATER MANAGEMENT

The proposed stormwater management system for the Beach Club Improvements project has been designed to meet the requirements of local, regional, and state stormwater ordinances and guidelines, including but not limited to those of the Town of Lewisboro and the NYSDEC. Specifically, the following codes / regulations have been used to design this SWPPP:

• NYSDEC SPDES General Permit for Stormwater Discharges from Construction Activities, General Permit GP-0-20-001 (GP-0-20-001).

Since the subject project proposes the disturbance of more than 1 acre, coverage under the New York State Department of Environmental Conservation (NYSDEC) SPDES General Permit for Stormwater Discharges from Construction Activities (GP-0-20-001) is required.

In order to meet the requirements set forth by GP-0-20-001, and the latest edition of the NYSDEC *New York State Stormwater Management Design Manual* (Design Manual), including the requirements listed in Chapter 9: *Redevelopment* (Chapter 9) and Chapter 10: *Enhanced Phosphorus Removal Standards* (Chapter 10) was referenced for the design of the proposed stormwater collection, conveyance and treatment system.

The Design Manual specifies five design criteria that are discussed in detail below. They are Runoff Reduction Volume ( $RR_v$ ), Water Quality Volume ( $WQ_v$ ), Stream Channel Protection Volume ( $CP_v$ ), Overbank Flood Control ( $Q_f$ ), and Extreme Storm Control ( $Q_p$ ). The first two requirements relate to treating water quality, while the later pertain to stormwater quantity (peak flow) attenuation.

To address stormwater quantity requirements of the NYSDEC, the "HydroCAD" Stormwater Modeling System," by HydroCAD Software Solutions LLC in Tamworth, New Hampshire, was used to model and assess the peak stormwater flows for the subject project. HydroCAD is a computer aided design program for modeling the hydrology and hydraulics of stormwater runoff. It is based primarily on hydrology techniques developed by the United States Department of Agriculture, Soil Conservation Service (USDA, SCS) TR-20 method combined with standard hydraulic calculations. For details on the input data for the subcatchments and design storms, refer to Appendices B through C and for the supporting data relative to the soils breakdown within the overall contributing area shown in the HydroCAD analysis, see Appendix B and C of this report:

The input requirements for the HydroCAD computer program are as follows:

Subcatchments (contributing watershed/sub-watersheds)

- Design storm rainfall in inches
- CN (runoff curve number) values which are based on soil type and land use/ground cover
- Tc (time of concentration) flow path information

Stormwater Pond

- Surface area at appropriate elevations
- Flood elevation
- Outlet structure information

The following is a general description of the input data used to calculate the pre- and post-development stormwater runoff values. For detailed information for each subcatchment and pond, see Appendices B&C. The precipitation values for the 1-Year, 10-Year, 100-Year 24-hour design storm events and rainfall distribution curves utilized for this report were obtained from the information provided by Northeast Regional Climate Center(NRCC) and the Natural Resources Conservation Service(NRCS) which is available online at *www.precip.eas.cornell.edu*. The values provided for all design storms analyzed are listed below.

24-Hour Rainfal		
2.82"		
5.07"		
9.02"		

The CN (runoff curve number) values utilized in this report were referenced from the USDA, SCS publication *Urban Hydrology for Small Watersheds*.

#### 2.1 Chapter 10: Enhanced Phosphorus Removal Standards

As noted above, the New York City East of Hudson Watershed has been identified in the SPDES General Permit GP-0-20-001 as a watershed requiring compliance with the Enhanced Phosphorus Removal Standards when post-construction stormwater management practices are proposed. Chapter 10 establishes four goals to meet sizing performance standards:

- Goal 1: Reducing Runoff Volumes
- Goal 2: Effective Bypass Treatment
- Goal 3: Achieving Effluent Concentrations for Particulate Phosphorus
- Goal 4: Achieving Effluent Concentrations for Dissolved Phosphorus

In order to achieve the first goal, the site design shall, "assess the feasibility of hydrological source controls and reduce the total water quality volume by source control, implementation of green infrastructure, or standard SMP's with RR_v capacity, according to the process defined in Chapters 3 and 4 of the Design Manual. Each plan must include a rationale for acceptance and rejection of the various controls." A discussion on RR_v can be found in section 2.2 below. Based upon the results of onsite soil testing, the soils onsite in select areas are not suitable for infiltration. Therefore, the use of a cistern (classified as a GIP with RRv capacity) has been maximized, to treat the stormwater runoff from the proposed impervious surfaces and satisfy RRv minimum requirements. As such, Goal 1 has been achieved in this SWPPP.

Goal 2 cites that proposed stormwater management practices should achieve less than 15% effective treatment bypass of the long-term runoff volume. Chapter 10 further notes this goal is satisfied by capturing and treating the 1-year 24-hour design storm. The NYSDEC stormwater quality treatment practices proposed for this have been designed in accordance with Chapter 10 by utilizing the 1-yr, 24-hour design storm to generate the  $WQ_v$  /  $RR_v$ . As such, Goal 2 has been achieved in this SWPPP.

Achieving effluent concentrations for particulate phosphorus, Goal 3, is satisfied by achieving an 80% net removal of particulate phosphorus for a median influent concentration of 0.5mg/l. Chapter 10 states that through designing proposed SMP's in accordance with Section 10.4 this goal will be achieved. The proposed stormwater pond has been designed in accordance with Section 10.4.1 of Chapter 10 thus satisfying the requirements of this goal.

Goal 4, achieving effluent concentration for dissolved phosphorus, is achieved by obtaining a 60% net removal of dissolved phosphorus given a median influent concentration of 0.15mg/l. As with Goal 3, Goal 4 is achieved by designing the proposed SMP's in accordance with Section 10.4 of Chapter 10. As noted above the proposed stormwater pond has been designed in accordance with section 10.4.1 of Chapter 10 thus satisfying the requirements of this goal.

#### 2.2 NYSDEC Runoff Reduction Volume (RRv)

The Runoff Reduction Volume ( $RR_v$ ) criterion is intended to replicate pre-development hydrology by maintaining preconstruction infiltration, peak flow runoff, discharge volume, as well as minimizing concentrated stormwater flow. As stated in Chapter 4 of the Design Manual,  $RR_v$  may be treated with standard stormwater management practices (SMP's) sized in accordance with the Chapter 4/6

requirements, or with green infrastructure practices (GIP's) sized in accordance with the requirements set forth for each practice in Chapter 5. This requirement has been achieved on the subject project providing a cistern designed as a GIP. Runoff reduction is achieved when runoff from a percentage of the impervious area on the site is captured, routed through a SMP or a GIP, infiltrated to the ground, reused, reduced by evapotranspiration, and eventually removed from the stormwater discharge from the site. Through this implementation, the design of the cistern as a GIP equal to 100% of the WQ_v, the RRv requirements will be achieved.

Section 4.3 of the Design Manual states for sites that do not achieve runoff reduction to preconstruction condition must, at a minimum reduce a percentage of the runoff from impervious areas to be constructed on the site a minimum  $RR_{\nu}$ . The following equation can be used to determine the minimum runoff reduction volume:

The minimum runoff reduction volume shall be  $RRv_{minimum} = \frac{(P)(R_v)(Ai)}{12}$ Where, S = Hydrologic Soil Group (HSG) Specific Reduction Factor

0	
A <i>i</i> c	= Total Area of New Impervious Cover
Ai	= Impervious cover targeted for Runoff Reduction
Bv	= (S)(A <i>i</i> c) = 0.95
	- 0.00

For detailed calculations of the runoff reduction for the proposed cistern, see Appendix A. Listed in Table 2.2.1 below is a summary of the NYSDEC compliant GIPs, and satisfaction of the NYSDEC RRv requirements.

The project proposes thirteen (13) new trees of 2-inch caliper or greater within close proximity to the proposed new impervious surfaces. In accordance with Chapter 5, an area reduction of 100 sf of new impervious area per new tree was used in the RRv calculations in Appendix A and the table below. A separate HydroCAD model is provided in Appendix A to equate the volume reduction associated with the thirteen (13) new trees or 1,300 sf of impervious area reduction. As shown in Appendix A, the runoff volume associated with 1,300 sf of impervious surface is 280 c.f., therefore the proposed tree plantings within close proximity to the new impervious surfaces provide 280 c.f. of RRv.

Subcatchment	RR _v <i>Required</i> = WQ _v (c.f.) From Table 2.3.1	RR _v <i>Minimum</i> (c.f.) Calculated in Appendix A	NYSDEC Practice Designation	Stormwater Management Practice ID	Storage Volume Provided below System Outlet (c.f.) (See Appendix C)	RRv Provided (C.f.)	Total RRv Provided (c.f.)
Design Line	2,008	848	Cistern	1.2P	705	705	985
Design Line			Tree Planting	-	-	280	900

#### Table 2.2.1 Runoff Reduction Volume Summary

As shown in the table above the RRv *provided* is greater than the RRv *Minimum* and RRv *Required*, therefore the RRv requirement has been met for the subject project.

The cistern is proposed to provide a portion of the non-potable irrigation water demand for the onsite landscaping. An irrigation system, designed by others, will be connected to the pump within the cistern to irrigate the onsite landscaping. The daily irrigation water demand for the proposed onsite landscaping is 2,135 gallons per day, or 4,270 gallons every two days, which exceeds the RRv minimum. This will allow the cistern to provide a storage volume greater than the RRv minimum between storm events in accordance with the Design Manual. Irrigation is proposed for the site landscaping including the proposed onsite trees, evergreens, shrubs, and ground cover plantings. A water demand of 10 gallons per day for every 1" in diameter of tree/evergreen tree is anticipated and 5 gallons per day per shrub. For

the ground cover areas, the recommended irrigation demand is 2.5-inch per week, which equates to 0.23 gallons per day per square foot of area. The anticipated daily water demand for the site landscaping is calculated below. As shown in the calculations below, the storage volume in the cistern exceeds the RRv minimum required for the cistern.

Cistern Sizing = 705 cf x 7.5 gallons/cf = 5,287 gallon cistern

 $RR_v$  minimum (minus credit from Tree Planting) = 568 cf x 7.5 gallons/cf = 4,260 gallons

Landscaping	Tree Caliper	Quantity	Water Demand per Plant (gpd)	Water Demand (gpd)
Shade/Flowering Trees	3"	21	30	630
Shaue/Flowening frees	2"	12	20	240
Evergreen Trees	2"	12	20	240
Shrubs	-	44	5	220
Ground Cover / Lawn	-	3,500	0.23 GPD/SF	805
	2,135			

Table 2.2.2 Daily Irrigation Water Demand Summary

2.3 NYSDEC Water Quality Volume (WQv)

For new impervious surfaces and as required by Chapter 10, the WQv shall be the runoff volume from the 1-year, 24-hour storm event generated by the subcatchment. As permitted by Chapter 9 of the Design Manual, the portions of the existing impervious surfaces being redeveloped with impervious surfaces will only require 25% of the WQ_v to be treated. Subcatchments 1.1S and 1.2S include redevelopment of existing impervious surfaces and the creation of new impervious surfaces within the same subcatchment. Appendix C has broken subcatchment 1.1S and 1.2S into two nodes so the redeveloped impervious surfaces contained within the subcatchment can be accounted for separately and the 25% reduction allowed by Chapter 9 applied. The calculation for the WQv for the areas of redevelopment are shown in the table below.

 Table 2.3.1 - Water Quality Volume Calculation for Redevelopment

Subcatchments	WQ _v ¹	Full WQ _v ²	25% WQv ³	WQv Initial ⁴
	New Impervious Surface	Redeveloped Impervious Surface	Redevelopment Calculation (c.f.)	(c.f.)
1.1S	(c.f.) 1,441	(c.f.) 259	65	1,506
1.2S	388	453	114	502

¹ Refer to Appendix C for the stormwater runoff volume from the 1-year, 24-hour storm event for the areas of new impervious surfaces within the subcatchment.

² Refer to Appendix C for the stormwater runoff volume from the 1-year, 24-hour storm event for the existing impervious areas proposed to be redeveloped within the subcatchment.

³ In accordance with Chapter 9 of the Design Manual, only 25% of the WQv from the existing impervious surfaces to be redeveloped requires treatment. The 25% reduction has been accounted for in the volumes provided.

⁴ The volumes provided are the sum of the stormwater runoff volume from the new impervious surfaces and 25% of the runoff volume from the existing impervious surfaces to be redeveloped. These volumes are used in the sizing calculations for the SMP's.

The stormwater pond and cistern has been sized in accordance with Chapter 5 and 6 of the Design Manual, as they have been sized to store the entire water quality volume (WQ_v) from the proposed improvements. The subject project is located in the New York City Watershed, which is listed as a phosphorus-limited watershed per the NYSDEC regulations. Therefore the stormwater management practices have been designed in general accordance with the Enhanced Phosphorus Removal

Supplement (Chapter 10) of the Design Manual. As outlined in Chapter 10, the treatment volume for the WQv is the runoff volume produced during the 1-year 24-hour design storm as calculated in Table 2.3.1 above. The cistern shall be used to irrigate the proposed landscaping in the area of the proposed Beach Club improvements.

The P-1 Micropool Extended Detention Pond has been sized in accordance with Chapter 6 of the Design Manual as shown in the table below. The pond has been sized to store a minimum of 20% of the WQv in the permanent pool and a maximum of 80% in extended detention. A forebay is provided at the pipe inflow to the pond and has been sized to provide greater than 10% of the WQv. The provided volume for the P-1 Pond can be verified in the stage storage tables contained in Appendix C. A P-1 Micropool Extended Detention Pond was chosen due to the overall contributing area to the proposed pond.

Design Elements	Required	Provided	Remarks
Pond Location	Not within Jurisdictional Waters	Outside of Jurisdictional Waters	See Project Plans
Forebay Volume	10% of WQv (151 cubic feet)	34% of WQv (510 cubic feet)	See Appendix C
Forebay Depth	4' Min. – 6' Max.	4' Provided	See Project Plans
WQv Storage	20% Min. within Permanent Pool (301 cubic feet)	100%+ within Permanent Pool (2,420 cubic feet)	See Appendix C
Minimum Length to Width Ratio	1.5 : 1	Greater than 1.5 : 1	See Project Plans
Minimum Surface Area to Drainage Area Ratio	1 : 100	1 : 15	See Project Plans
Benches at Water Level	Aquatic Bench	Aquatic Bench	See Project Plans
Landscaping	Pond and Buffer Plantings Required	Pond and Buffer Plantings Provided	See Project Plans

Table 2.1.3 P-1 Micropool Extended Detention Pond Summary

It should be noted that the stormwater pond and cistern have been sized to treat 100% of the water quality volume to meet the requirements set forth in the Design Manual. By meeting the Water Quality Volume requirements through employment of the pond and cistern, the water quality objectives of the NYSDEC has been met.

### 2.4 NYSDEC Stream Channel Protection Volume, CPv

The Stream Channel Protection (CP_v) criterion is intended to protect stream channels from erosion and is accomplished by the 24-hour extended detention of the center-of-mass of the 1-year, 24-hour storm event or by fully infiltrating the stormwater runoff from the 1-year, 24-hour storm event. As permitted by Chapter 9, Section 9.2 of the Design Manual, for areas of redevelopment activities, the CP_v criterion is not required if the peak flow for the project site in the post-development condition is less than the predevelopment condition for the 1-year 24-hour storm event. As shown in Table 2.5.1 below, the project proposes a decrease in the peak flow from the pre to post-development condition for the 1-year, 24-hour storm event. It should be noted, the proposed P-1 Pond has been designed to provide 24-hour extended detention of the 1-year, 24-hour storm event from the tributary area. As shown in the HydroCAD model in Appendix C, the pond 1.1P provides greater than 1,440 minutes (24-hours) center-of-mass detention time during the 1-year, 24-hour storm event.

### 2.5 NYSDEC Overbank Flood Control, Qp, and Extreme Flood Control, Qf

The Overbank Flood Control  $(Q_p)$  requirement is intended to prevent an increase in the frequency and magnitude of out-of-bank flooding events generated by urban development. Overbank control requires storage to attenuate the post-development 10-year, 24-hour peak discharge to pre-development rates. The Extreme Flood Control  $(Q_f)$  requirement is intended to prevent the increased risk of flood damage from large storm events, maintain the boundaries of the pre-development 100-year flood plain, and protect the physical integrity of stormwater management practice. Extreme flood control requires storage to attenuate the post-development 100-year, 24-hour peak discharge to pre-development rates. As shown in Table 2.5.1 attenuation for both the 10-year and 100-year 24-hour storms has been provided thus satisfying the  $Q_p$  and  $Q_f$  requirements.

24-HOUR DESIGN STORM PEAK FLOWS (c.f.s.)						
1-YEAR 10-YEAR 100-YEAR (Overbank Flood Control) (Extreme Flood Control)						
	Pre	Post	Pre Post		Pre	Post
Design Line 1	3.09	2.97	13.07	12.85	32.90	32.39

Table 2.5.1– Existing and Proposed Conditions Peak Flows

As shown in the above table the peak flows from the contributing areas to Design Line 1 in the post development condition has been mitigated to below the existing condition levels, thus meeting the general requirements of the NYSDEC.

As requested by the NYCDEP, the following table includes a summary of the runoff volumes in the pre and post-development condition to Design Line 1 for the 1-year, 10-year, and 100-year, 24-hour storm event.

1							
	24-HOUR DESIGN STORM RUNOFF VOLUME (a.f.)						
	1-YEAR 10-YEAR 100-YEAR						<b>EAR</b>
		Pre	Post	Pre	Post	Pre	Post
	Design Line 1	0.494	0.517	1.777	1.816	4.701	4.762

Table 2.5.2– Existing and Proposed Conditions Runoff Volumes

### 3.0 STORMWATER CONVEYANCE SYSTEM

The project proposes to install a new stormwater collection and conveyance system to convey the stormwater runoff from the redeveloped areas to the proposed stormwater management practices. The proposed stormwater collection and conveyance system for the project consists of precast concrete drainage structures and HDPE drainage piping. The proposed stormwater conveyance system has been sized utilizing the Rational Method and is a standard method used by engineers to develop flow rates for sizing collection systems. The Rational Method calculates flows based on a one-hour design storm. The proposed collection system has been sized to convey, at a minimum, the 100-year design storm. Sizing calculations for the proposed stormwater conveyance piping are provided in Appendix G.

### 4.0 EROSION AND SEDIMENT CONTROL

Erosion and sediment control should be accomplished by four basic principles: diversion of clean water, containment of sediment, treatment of dirty water, and stabilization of disturbed areas. Diversion of clean water should be accomplished with swales. This diverted water should be safely conveyed around the construction area as necessary and discharged downstream of the disturbed areas. Sediment should be contained with the use of silt fence at the toe of disturbed slopes. Disturbed areas should be permanently stabilized within 7 days of final grading to limit the required length of time that the temporary facilities must be utilized. The owner will be responsible for the maintenance of the temporary erosion control facilities. Refer to the Project Drawings for further information implementation of the Erosion Control Plan and Construction Sequence.

4.1 Temporary Erosion and Sediment Control Facilities

Temporary erosion and sediment control facilities should be installed and maintained as required to reduce the impacts to off-site properties. The owner will be required to provide maintenance for the

temporary erosion and sediment control facilities. In general, the following temporary methods and materials should be used to control erosion and sedimentation from the project site:

- Stabilized Construction Entrance
- Silt Fence Barriers
- Storm Drain Inlet Protection
- Temporary Soil Stabilization
- Temporary Sediment Trap

All temporary erosion control measures shall be maintained in accordance with the Erosion & Sediment Control Maintenance Schedule contained on the Project Drawings, and as discussed below.

A stabilized construction entrance should be installed at the site entrance as shown on the project plans. The design drawings will include details to guide the contractor in the construction of this entrance. The intent of the stabilized construction entrance is to prevent the "tracking" of soil from the site. Dust control should be accomplished with water sprinkling trucks if required. During dry periods, sprinkler trucks should wet all exposed earth surfaces as required to prevent the transport of air-borne particles to adjoining areas.

Siltation barriers constructed of geosynthetic filter cloth should be installed at the toe of all disturbed slopes. The intent of these barriers is to contain silt and sediment at the source and inhibit its transport by stormwater runoff. The siltation barriers will also help reduce the rate of runoff by creating filters through which the stormwater must pass. During construction, the siltation barriers shall be inspected weekly and after a rainfall event and shall be cleaned/replaced when needed.

Storm drain inlet protection in the form of filter fabric inlet protection will be installed around all proposed inlets. The filter fabric inlet protection will serve to filter stormwater runoff before it enters the collection system. Throughout construction the concrete drainage structures, associated piping and inlet protections shall be inspected weekly and after a rainfall event. These items shall be cleaned, repaired and/or replaced when needed.

When land is exposed during development, the exposure shall be kept to the shortest practical period, but in no case more than 7 days. Temporary grass seed and mulch shall be applied to any construction area idle for two weeks. The temporary seeding and mulching shall be performed in accordance with the seeding notes illustrated on the project drawings. Disturbance shall be minimized in the areas required to perform construction. Upon completion of final grading topsoil, permanent seeding and mulch shall be applied in accordance with the project drawings.

The P-1 Micropool Extended Detention Pond will act as temporary sediment trap during construction of the site. The stormwater runoff from disturbed areas will be directed to the sediment trap. The sediement trap will be sized in accordance with the New York State Standards and Specifications for Erosion and Sediment Control. Sizing calculations for the temporary sediement trap are provided in Appendix I.

The stormwater runoff will be managed by the temporary erosion and sediment control facilities during construction. As discussed in the construction sequences provided the project plans the stabilized construction entrance shall be installed at the site entrance and silt fence shall be installed along the down hill perimeter of where soil disturbing activities will occur containing sediment laden stormwater runoff on-site.

### 4.2 Permanent Erosion and Sediment Control Facilities

Permanent erosion and sediment control will be accomplished by diverting stormwater runoff from steep slopes, controlling/reducing stormwater runoff velocities and volumes, and vegetative and structural surface stabilization. All of the permanent facilities are relatively maintenance free and only require periodic inspections. The owner will provide maintenance for all the permanent erosion and sediment control facilities.

Rock outlet protection will be provided at the discharge end of all piped drainage systems, and will be sized in accordance with the Blue Book. The purpose of the rock outlet protection is to reduce the depth, velocity, and energy of water, such that the flow will not erode the receiving downstream reach. The rock outlet protection shall be inspected for evidence of scour beneath the riprap and/or for any dislodged stones. Inspections of the rock outlet protection shall be performed during the inspections of the post-construction SMP's for the project.

Other than paved surfaces, disturbed surfaces will be stabilized with vegetation within 10 days of final grading. Permanent seed mix and mulch shall be applied to idle areas to minimize the amount of exposed soil. Permanent seed mixtures are proposed for the project and illustrated on project drawings. Application rates for the seed and mulch are provided on the project drawings. The vegetation will control stormwater runoff by preventing soil erosion, reducing runoff volume and velocities, and providing a filter medium. Permanent seeding should optimally be undertaken in the spring from March 21st through May 20th and in late summer from August 15th to October 15th.

### 5.0 IMPLEMENTATION AND MAINTENANCE

#### 5.1 Construction Phase

Details associated with the implementation and maintenance of the proposed stormwater facilities and erosion control measures during construction are shown on the project drawings. Soil disturbance for the subject project shall not exceed five acres at any given time. The erosion control plan will include associated details and notes to aid the contractor in implementing the plan. Construction is anticipated to begin in the fall of 2022, and anticipated to be completed by the spring of 2023.

During construction, a Site Log Book, Appendix D, is required to be kept per NYSDEC SPDES General Permit GP-0-20-001. Erosion and sediment control inspections are required to be conducted as necessary under coverage of the permit (minimum twice a week) and an updated logbook and a copy of the SWPPP is required to be kept on site for the duration of the construction activities. The Construction Site Log Book is an appendix taken from the *New York Standards and Specifications for Erosion and Sediment Control* (Blue Book).

In addition to the proposed erosion and sediment control facilities, the following good housekeeping best management practices shall be implemented to mitigate potential pollution during the construction phase of the project. The general contractor overseeing the day-to-day site operation shall be responsible for the good housekeeping best management practices included in the following general categories:

- Material Handling and Waste Management
- Establishment of Building Material Staging Areas
- Establishment of Washout Areas
- Proper Equipment Fueling and Maintenance Practices
- Spill Prevention and Control Plan

All construction waste materials shall be collected and removed from the site regularly by the general contractor. The general contractor shall supply waste barrels for proper disposal of waste materials. All personnel working on the site shall be instructed of the proper procedures for construction waste disposal.

Although it is not anticipated any hazardous waste materials will be utilized during construction, any hazardous waste materials shall be disposed of in accordance with federal, state, and local regulations. No hazardous waste shall be disposed of on-site. Hazardous waste materials shall be stored in appropriate and clearly marked containers and segregated from the other non-waste materials. All hazardous waste shall be stored in a structurally sound and sealed shipping containers located in the staging areas. Material safety data sheets, material inventory, and emergency contact numbers will be maintained in the office trailer. All personnel working on the site shall be instructed of the proper procedures for hazardous waste disposal.

Temporary sanitary facilities (portable toilets) shall be provided on site during the entire length of construction. The sanitary facilities shall be located in the project staging area, or in an alternate area away from the construction activities on the site. The portable toilets shall be inspected weekly for evidence of leaking holding tanks.

All recyclables, including wood pallets, cardboard boxes, and all other recyclable construction scraps shall be disposed of in a designated recycling barrel provided by the contractor and removed from the site regularly. All personnel working on the site shall be instructed of the proper procedures for construction waste recycling.

All construction equipment and maintenance materials shall be stored in a construction staging area. Silt fence shall be installed down gradient of the construction staging area. Shipping containers shall be utilized to store hand tools, small parts, and other construction materials, not taken off site daily. Construction waste barrels, recycling barrels and if necessary hazardous waste containers shall be located within the limits of the construction staging area.

Throughout the construction of the project, several types of vehicles and equipment will be used on-site. Fueling of the equipment shall occur within the limits of the construction staging area. Fuel will be delivered to the site as needed, by the general contractor, or a party chosen by the general contractor. Only minor vehicle equipment maintenance shall occur on-site, all major maintenance shall be performed off-site. All equipment fluids generated from minor maintenance activities shall be disposed of into designated drums and stored in accordance with the hazardous waste storage as previously discussed.

The designated temporary concrete washout areas shall be constructed in accordance with the detail in the general locations as shown on the project plans. The temporary concrete washout areas shall be lined with plastic sheeting as specified on the detail free of holes or tears. Should the liner rip or tear at any time it shall be replaced immediately. All concrete mixer trucks and chutes shall be washed in the designated concrete wash areas. All personnel working on the site including concrete equipment operators shall be instructed of the locations and proper procedures for concrete washout. When the temporary concrete washout areas are no longer needed the hardened concrete and materials used to construct the washout area shall be broken up and removed from the site and disposed of in a landfill.

Vehicles and equipment shall be inspected on each day of use. Any leak discovered shall be repaired immediately. All leaking equipment unable to be repaired shall be removed from the site. Ample supplies of absorbent, spill-cleanup materials, and spill kits shall be located in the construction staging area. All spills shall be cleaned up immediately upon discovery. Spent absorbent materials and rags shall be hauled off-site immediately after the spill is cleaned for disposal at a local landfill. All personnel working on the site shall be instructed of the proper procedures for spill prevention and control. Any spill large enough to discharge to surface water will be immediately reported to the local fire / police departments, NYCDEP, and the National Response Center 1-800-424-8802.

Vegetation should be inspected every 30 days and after every major storm event until established, after which inspections should take place on a quarterly basis and after every large storm event. Damaged areas should be immediately re-seeded and re-mulched.

### 5.2 Soil Restoration

Soil Restoration is required to be applied across areas of the development site where soils have been disturbed and will be vegetated. The purpose is to recover the original properties and porosity of the soil compacted during construction activity. Soil Restoration is applied in the cleanup, restoration, and landscaping phase of construction followed by the permanent establishment of an appropriate, deeprooted groundcover to help maintain the restored soil structure. Soil restoration includes mechanical decompaction and compost amendment. The table below describes various soil disturbance activities related to land development, soil types and the requirements for soil restoration for each activity as identified in the Design Manual. Restoration is applied across areas of a development site where soils have been compacted and will be vegetated according to the criteria defined in the following table:

(Onsite soils within	<b>Soil Restoration Requirements</b> ^{1, 2,4} (Onsite soils within the limit of disturbance belong to Hydrologic Soil Groups (HSG) B & C)						
Type of Soil Disturbance		on Requirement	Comments/Examples				
No soil disturbance	Restoration	not permitted	Preservation of Natural Features				
Minimal soil disturbance	Restoration	not required	Clearing and grubbing				
Areas where topsoil is	HSG A & B	HSG C&D	Protect area from any ongoing				
stripped only - no change in grade	Apply 6 inches of topsoil	Aerate ³ and apply 6 inches of topsoil	construction activities.				
	HSG A &B	HSG C&D					
Areas of cut or fill	Aerate1 and apply 6 inches of topsoilApply full Soil Restoration 2						
Heavy traffic areas on site (especially in a zone 5-25 feet around buildings but not within a 5-foot perimeter around foundation walls)	Apply full Soil Restoration (decompaction and compost Enhancement ⁶ )						
Areas where Runoff Reduction and/or Infiltration practices are applied	Restoration not required, but may be applied to enhance the reduction specified for appropriate practices.		Keep construction equipment from crossing these areas. To protect newly installed practice from any ongoing construction activities construct a single phase operation fence area				
Redevelopment projects	Soil Restoration is required on redevelopment projects in areas where existing impervious area will be converted to pervious area.						

1. Aeration includes the use of machines such as tractor-drawn implements with coulters making a narrow slit in the soil, a roller with many spikes making indentations in the soil, or prongs which function like a mini-subsoiler.

2. Per "Deep Ripping and De-compaction, DEC 2008".

3. Aeration includes the use of machines such as tractor-drawn implements with coulters making a narrow slit in the soil, a roller with many spikes making indentations in the soil, or prongs which functions like a mini-subsoiler.

- 4. During periods of relatively low to moderate subsoil moisture, the disturbed soils are returned to rough grade and the following Soil Restoration steps applied:
  - 5.1. Apply 3 inches of compost over subsoil.
  - 5.2. Till compost into subsoil to a depth of at least 12 inches using a cat-mounted ripper, tractor-mounted disc, or tiller, mixing, and circulating air and compost into subsoils.
  - 5.3. Rock-pick until uplifted stone/rock materials of four inches and larger size area cleaned off the site.
  - 5.4. Apply topsoil to a depth of 6 inches.
  - 5.5. Vegetate as required by seeding notes located on the project drawings.
  - 5.6. Tilling should not be performed within the drip line of any existing trees or over any utility installations that are within 24 inches of the surface.
- 6. Compost shall be aged, from plant derived materials, free of viable weed seeds, have no visible free water or dust produced when handling, pass through a half inch screen and have a pH suitable to grow desired plants.

After soil restoration is completed an inspector should be able to push a 3/8" metal bar twelve inches into the soil with just body weight. Following decompaction/soil restoration activities, the following maintenance is anticipated during the first year:

- Initial inspections for the first six months (once after each storm greater than a half-inch).
- Reseeding to repair bare or eroding areas to assure grass stabilization.

- Water once every three days for first month, and then provide a half inch of water per week during first year. Irrigation plan may be adjusted according to the rain event.
- Fertilization may be needed in the fall after the first growing season to increase plant vigor.

In order to ensure the soil remains decompacted the following ongoing maintenance is recommended:

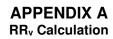
- Planting the appropriate ground cover with deep roots to maintain the soil structure.
- Keeping the site free of vehicular and foot traffic or other weight loads. Consider pedestrian footpaths (sometimes it may be necessary to de-thatch the turf every few years).

#### 5.3 Long Term Maintenance Plan

The stormwater facilities for the subject project have been designed to minimize the required maintenance. This section discusses the minimum maintenance requirements to insure long-term performance of the stormwater facilities. Initially the stormwater facilities will require an increased maintenance and inspection schedule until all portions of the site are stable. Generally the stormwater facilities consist of either collection and conveyance components or treatment components.

The stormwater collection and conveyance system is composed of HDPE drainage pipe and precast concrete drainage structures. The owner will assume the maintenance responsibilities for the drainage system. Minimal maintenance is typically required for these facilities. All pipes should be checked for debris and blockages and cleaned as required. All drain inlet sumps shall be cleaned to removed deposited sediment. During the cleaning process, the pipes should be inspected for structural integrity and overall condition; repairs and/or replacement should be made as required.

Additionally, the stormwater management practices including the stormwater pond and cistern shall be checked for deposited sediment as well. Inspection and maintenance requiredments for the proposed stormwater management practices per the Design Manual are provided in Appendix F of this report and on the project plans.



### **RRv Calculation Worksheet - Design Line**

	N	S	/	<b>7</b>	E
ENG. LAND	INEER SCAPE	NG, ARCH	SUR	VEYI TURE,	NG & . P.C.

Project:	Waccabuc Country Club - Beach Club Improvements
Project #:	20228.100
Date:	2/7/2023

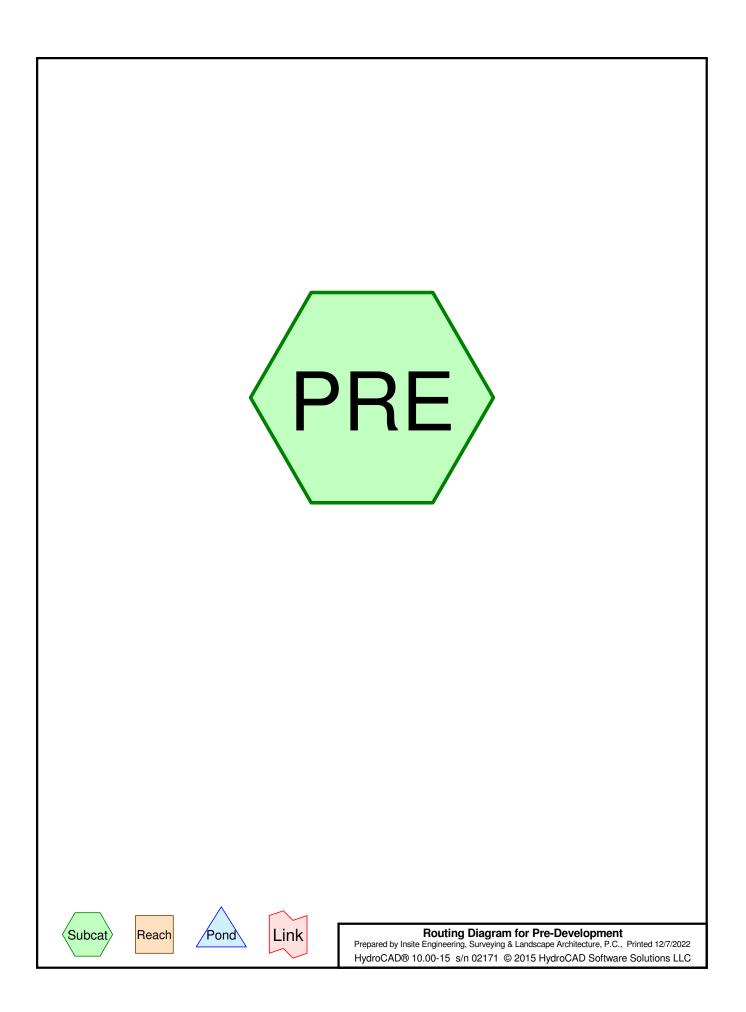
	er Quality Volume (WQv)	0.046 ac-ft	=	2,008 c.f.
(refer to Table 2.3.1	for Water Quality Volume)			
2. RRv Minimum =	[ (P) (Rv) (S) (Aic)] /12 wh	ere		
	P = Rainfall (in.)		=	2.82 in.
	Rv = 0.05 + 0.009 (100%)		=	0.95
	S = Hydrologic Soil Group S	pecific Reduction Factor	=	0.40
	[HSG A = 0.55] [HSG	B = 0.40] [HSG C = 0.30] [HSG D = 0.20]		
	Aic = Total area of new impe	ervious cover	=	0.22 Acres
	RRv Minimum		=	848 c.f.
3. RRv Required = F	RRv Initial - Green Infrastructure	e Practice (GIP) with Area Reduction		
GIP with	Area Reduction Applied in Proje	<u>ect</u>		
5.3.1 Cor	nservation of Natural Area		N/A	L .
5.3.2 She	eet Flow to Riparian Buffers or F	Filter Strips	N/A	L .
5.3.4 Tre	e Planting / Tree Box			280 c.f.
5.3.5 Dis	connection of Rooftop Runoff			-
5.3.6 Stre	eam Daylighting		N/A	L Contraction of the second seco
RRv Req	uired(=WQv-RRV by area)(Ref	er to HydroCAD output in this Appendix)	=	1,728 c.f.

GIP with Volume Reduction Applied in Project	WQv Treated (c.f.)	% of WQv Applied to <i>RRv</i> <i>Provided</i>	RRv Provideo (c.f.)
5.3.3 Vegetated Open Swales	. ,	20%	0
[HSG A / B = $20\%$ ] [HSG C / D = $10\%$ ] {Modified HSG C - D = $15\%$ - $12\%$ ]		10%	0
5.3.7 Rain Garden		40%	0
[No underdrains / Good Soils = 100%] [With underdrains / Poor Soils = 40%]			
5.3.8 Green Roof		100%	0
[RRv provided equals volume provided in Green Roof]			
5.3.9 Stormwater Planters		45%	0
[Infiltration Planters = 100%] [Flow Through HSG C = 45%] [Flow Though HSG D = 30%]			
5.3.10 Rain Tank / Cisterns	705	100%	705
5.3.11 Porous Pavement		100%	0
Infiltration Practice (Standard SMP)		100%	0
Bioretention Practice (Standard SMP)		40%	0
[Without Underdrains HSG A/B = 80%] [With Underdrain HSG C\D = 40%]			
Dry Swale (Open Channel Practice) (Standard SMP)		20%	0
[HSG A/B = 40%] [HSG C/D = 20%]			
RRv Provided =			705

5. Summary

RRv Initial	=	2,008 c.f.	
RRv Required	=	1,728 c.f.	
RRv Minimum	=	848 c.f.	
RRv Provided	=	985 c.f.	
WQv Required for Downstream SMP	=	743 c.f.	(= RRv Required - RRv Provided)
Is RRv Provided greater than or equal to RRv Minimum?		Yes	

### APPENDIX B Pre Development Computer Data



Pre-Development	NY-Waccabuc 24-hr S0P 1-yr Rainfall=2.82			
Prepared by Insite Engineering, Surveying & Lar	ndscape Architecture, P.C.	Printed 12/7/2022		
HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD So	•	Page 2		

# Summary for Subcatchment PRE:

Runoff = 3.09 cfs @ 12.42 hrs, Volume= 0.494 af, Depth= 0.54"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 1-yr Rainfall=2.82"

	Area	(ac) C	N Dese	cription		
	8.	200	70 Woo	ds, Good,	HSG C	
*	0.	250 9	98 Pave	ed parking		
*	0.	300 9	96 Grav	el surface		
	2.	300 !	55 Woo	ds, Good,	HSG B	
	11.	050 6	58 Wei	ghted Aver	ade	
	10.800 97.74% Pervious Area					
		250	2.26	% Impervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
	14.4	100	0.0500	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.40"
	6.9	550	0.0700	1.32		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	23	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.5	51	0.1000	1.58		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	7.8	811	0.1200	1.73		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	29.7	1,535	Total			

Pre-Development	NY-Waccabuc 24-hr S0P 1	0-yr Rainfall=5.07"
Prepared by Insite Engineering, Surveying & L	Landscape Architecture, P.C.	Printed 12/7/2022
HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD	Software Solutions LLC	Page 3

# Summary for Subcatchment PRE:

Runoff = 13.07 cfs @ 12.38 hrs, Volume= 1.777 af, Depth= 1.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 10-yr Rainfall=5.07"

	Area	(ac) C	N Dese	cription		
	8.	200	70 Woo	ds, Good,	HSG C	
*	0.	250 9	98 Pave	ed parking		
*	0.	300 9	96 Grav	el surface		
	2.	300 !	55 Woo	ds, Good,	HSG B	
	11.	050 6	58 Wei	ghted Aver	ade	
	10.800 97.74% Pervious Area					
		250	2.26	% Impervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	•
	14.4	100	0.0500	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.40"
	6.9	550	0.0700	1.32		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	23	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.5	51	0.1000	1.58		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	7.8	811	0.1200	1.73		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	29.7	1,535	Total			

Pre-DevelopmentNY-Waccabuc 24-hr S0P 100-yr Rainfall=9.02"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 12/7/2022HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 4

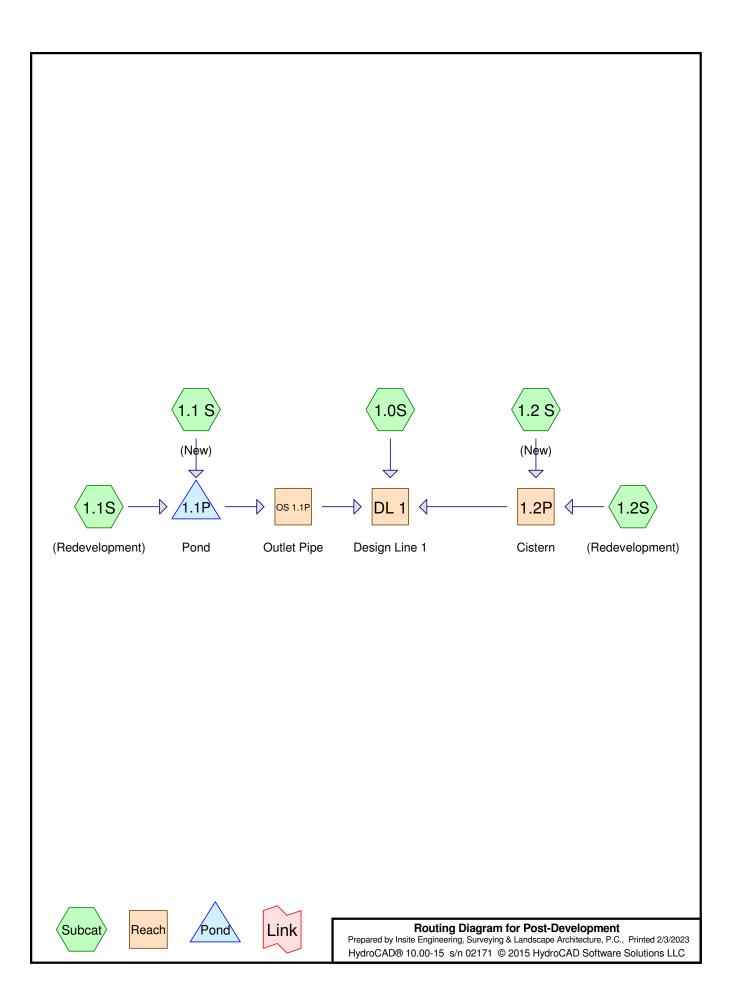
### Summary for Subcatchment PRE:

Runoff = 32.90 cfs @ 12.37 hrs, Volume= 4.701 af, Depth= 5.11"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 100-yr Rainfall=9.02"

	Area	(ac) C	N Dese	cription		
	8.	200	70 Woo	ds, Good,	HSG C	
*	0.	250	98 Pave	ed parking		
*	0.	300 9	96 Grav	el surface		
				ds, Good,	HSG B	
_	11.	050		ghted Aver		
	10.800 97.74% Pervious Area					
	0.250 2.26% Impervious Area					
				, •p •		
	Тс	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	14.4	100	0.0500	0.12		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.40"
	6.9	550	0.0700	1.32		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	0.1	23	0.0400	4.06		Shallow Concentrated Flow,
						Paved Kv= 20.3 fps
	0.5	51	0.1000	1.58		Shallow Concentrated Flow,
						Woodland Kv= 5.0 fps
	7.8	811	0.1200	1.73		Shallow Concentrated Flow,
_						Woodland Kv= 5.0 fps
	29.7	1,535	Total			

### APPENDIX C Post Development Computer Data



Post-Development	NY-Waccabuc 24-hr S0P	1-yr Rainfall=2.82"
Prepared by Insite Engineering, Surveying & Lands	scape Architecture, P.C.	Printed 2/3/2023
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# Summary for Subcatchment 1.0S:

Runoff = 2.93 cfs @ 12.42 hrs, Volume= 20,433 cf, Depth= 0.54"

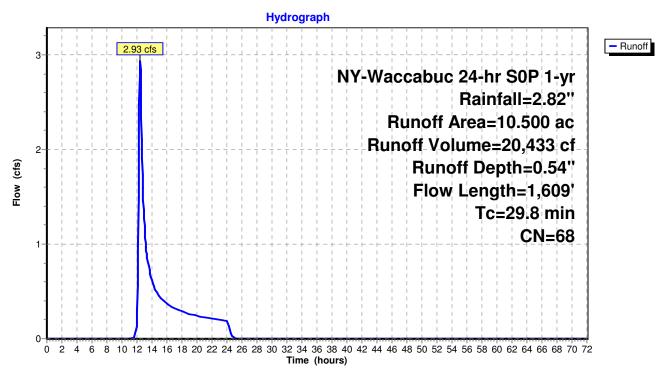
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 1-yr Rainfall=2.82"

Area	(ac) C	N Dese	cription		
7.	.800 7	70 Woo	ds, Good,	HSG C	
* 0.	.220 9	98 Pave	ed parking		
0.	.310 9	96 Grav	el surface	, HSG C	
2.	.120 5		ds, Good,		
0.	.050 6	61 >75°	% Grass c	over, Good	, HSG B
10.	.500 6	8 Weig	ghted Avei	age	
10.	.280	97.9	0% Pervio	us Area	
0.	.220	2.10	% Impervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
14.4	100	0.0500	0.12		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.40"
6.9	550	0.0700	1.32		Shallow Concentrated Flow,
					Woodland Kv= 5.0 fps
0.1	23	0.0400	4.06		Shallow Concentrated Flow,
0.5	- 1	0 1 0 0 0	4 50		Paved Kv= 20.3 fps
0.5	51	0.1000	1.58		Shallow Concentrated Flow,
E /	ECE	0 1000	1 70		Woodland Kv= 5.0 fps
5.4	565	0.1200	1.73		Shallow Concentrated Flow,
0.0	32	0.1340	16.61	13.04	Woodland Kv= 5.0 fps Pipe Channel,
0.0	52	0.1340	10.01	15.04	12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
0.2	268	0.1460	21.79	26.74	
0.2	200	0.1400	21.75	20.74	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.012
2.3	20	0.2000	0.15		Sheet Flow,
2.0	20	5.2000	0.10		Woods: Light underbrush n= 0.400 P2= 3.40"
29.8	1.609	Total			

29.8 1,609 Total

### **Post-Development**

Subcatchment 1.0S:



### Summary for Subcatchment 1.1 S: (New)

Runoff = 0.50 cfs @ 12.04 hrs, Volume= 1,441 cf, Depth= 0.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 1-yr Rainfall=2.82"

	Δr	ea (sf)	CN	Description			
	7 1	7,700			ing, HSG E	3	
		11,600				ood, HSG B	
		19,300		Weighted A			
		11,600			rvious Area		
		7,700		39.90% imp	pervious Ar	ea	
(n	Tc nin)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	5.0					Direct Entry,	
					Subcatch	nment 1.1 S: (New)	
						ograph	
	0.55		0.50 cfs			·	- Runoff
	0.5		-''		$\frac{1}{\frac{1}{1}} = \frac{1}{\frac{1}{1}} = \frac{1}{\frac{1}{1}} = \frac{1}{\frac{1}{1}} = \frac{1}{\frac{1}{1}} = \frac{1}{\frac{1}{1}}$		
	0.45-				$\begin{array}{cccccccccccccccccccccccccccccccccccc$	NY-Waccabuc 24-hr S0P 1-yr	
	-					Rainfall=2.82"	
	0.4-		-¦¦ <mark>-</mark> ¦	$-\frac{1}{1} - \frac{1}{1} - \frac{1}{1} - \frac{1}{1} - \frac{1}{1} - \frac{1}{1}$	$\frac{1}{1} = \frac{1}{1} = \frac{1}{1} = \frac{1}{1} = \frac{1}{1} = \frac{1}{1}$	Runoff Area=19,300 sf	
	0.35				$\begin{array}{cccccccccccccccccccccccccccccccccccc$	Runoff Volume=1,441 cf	
cfs)	0.3-					Runoff Depth=0.90"	
Flow (cfs)	-					Tc=5.0 min	
Ē	0.25		-''           				
	0.2			- $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ $ +$ -	$\begin{array}{c} 1 \\ + \\ - \\ + \\ 1 \end{array} \begin{array}{c} 1 \\ + \\ - \\ + \\ - \\ + \\ - \\ + \\ - \\ - \\ -$	·	
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# Post-DevelopmentNY-Waccabuc 24-hr SOP 1-yrRainfall=2.82"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 5

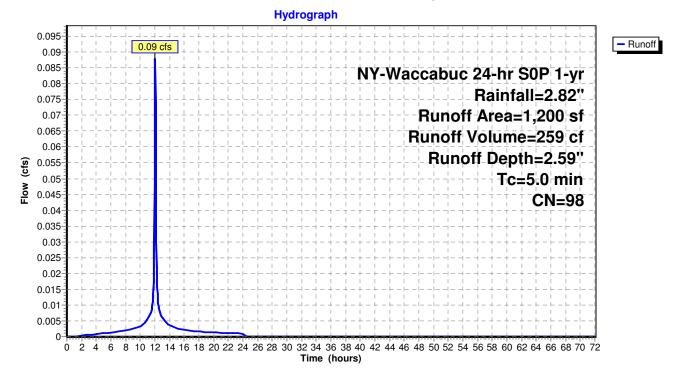
### Summary for Subcatchment 1.1S: (Redevelopment)

Runoff = 0.09 cfs @ 12.03 hrs, Volume= 259 cf, Depth= 2.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 1-yr Rainfall=2.82"

Area (sf	) CN	Description	l	
1,200	) 98	Paved park	ting, HSG B	3
1,200	)	100.00% In	npervious A	Area
Tc Leng (min) (fee		,	Capacity (cfs)	Description
5.0				Direct Entry,

### Subcatchment 1.1S: (Redevelopment)

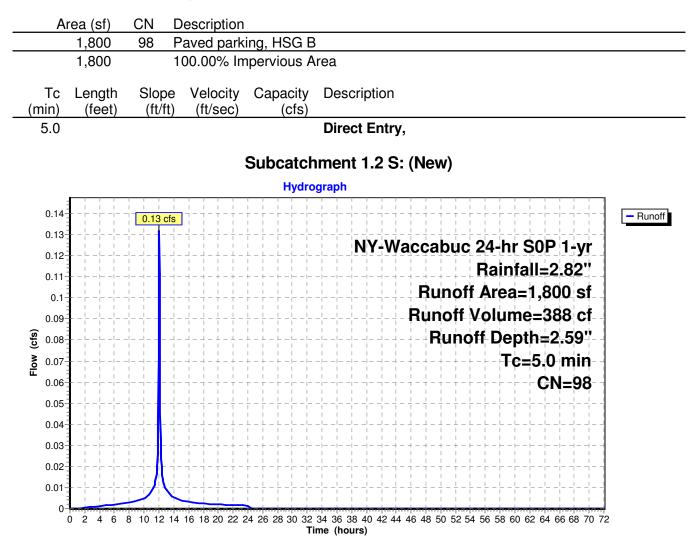


# Post-DevelopmentNY-Waccabuc 24-hr SOP 1-yrRainfall=2.82"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 6

### Summary for Subcatchment 1.2 S: (New)

Runoff = 0.13 cfs @ 12.03 hrs, Volume= 388 cf, Depth= 2.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 1-yr Rainfall=2.82"



# Post-DevelopmentNY-Waccabuc 24-hr SOP 1-yrRainfall=2.82"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15s/n 02171© 2015 HydroCAD Software Solutions LLCPage 7

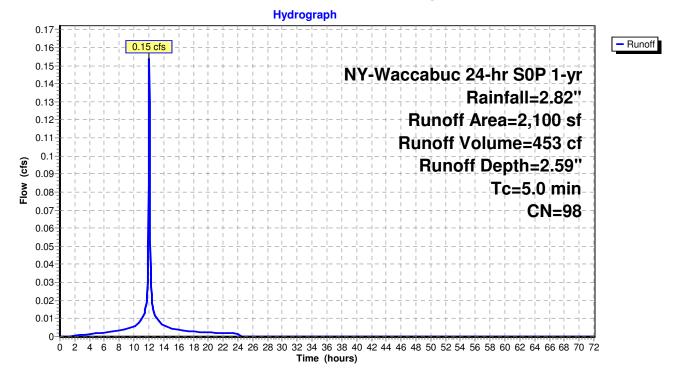
### Summary for Subcatchment 1.2S: (Redevelopment)

Runoff = 0.15 cfs @ 12.03 hrs, Volume= 453 cf, Depth= 2.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 1-yr Rainfall=2.82"

A	rea (sf)	CN I	Description		
	2,100	98 I	Paved park	ing, HSG B	3
	2,100	-	100.00% In	npervious A	vrea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

### Subcatchment 1.2S: (Redevelopment)



Post-Development	NY-Waccabuc 24-hr S0P	1-yr Rainfall=2.82"
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# Summary for Reach 1.2P: Cistern

Inflow Area =	0.090 ac,100.00% Impervious, Inflow	Depth = 2.59" for 1-yr event
Inflow =	0.29 cfs @ 12.03 hrs, Volume=	841 cf
Outflow =	0.29 cfs @ 12.03 hrs, Volume=	841 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Hydrograph Inflow Outflow 0.3 0.29 cfs 0.28 Inflow Area=0.090 ac 0.26 0.24 0.22 0.2 0.18 Flow (cfs) 0.16 0.14 0.12 0.1 0.08 0.06 0.04 0.02 0 0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

### Reach 1.2P: Cistern

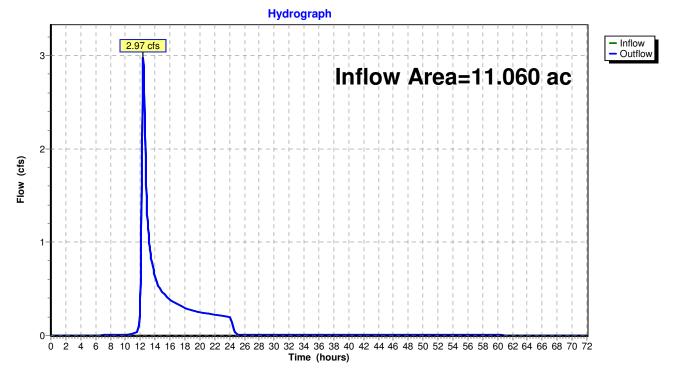
Post-Development	NY-Waccabuc 24-hr S0P	1-yr Rainfall=2.82"
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# Summary for Reach DL 1: Design Line 1

Inflow Area =	11.060 ac,	4.65% Impervious, In	flow Depth > 0.56"	for 1-yr event
Inflow =	2.97 cfs @	12.42 hrs, Volume=	22,518 cf	
Outflow =	2.97 cfs @	12.42 hrs, Volume=	22,518 cf, Atten=	0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

### Reach DL 1: Design Line 1



**Post-Development** NY-Waccabuc 24-hr SOP 1-yr Rainfall=2.82" Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C. Printed 2/3/2023 HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLC Page 10

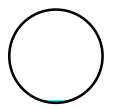
### Summary for Reach OS 1.1P: Outlet Pipe

Inflow Area = 0.471 ac, 43.41% Impervious, Inflow Depth > 0.73" for 1-yr event Inflow 0.01 cfs @ 24.06 hrs, Volume= 1.243 cf = Outflow 0.01 cfs @ 24.07 hrs, Volume= 1,243 cf, Atten= 0%, Lag= 0.7 min =

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 0.95 fps, Min. Travel Time= 0.9 min Avg. Velocity = 0.85 fps, Avg. Travel Time= 1.0 min

Peak Storage= 0 cf @ 24.07 hrs Average Depth at Peak Storage= 0.03' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.86 cfs

12.0" Round Pipe n= 0.012 Length= 50.0' Slope= 0.0100 '/' Inlet Invert= 476.50', Outlet Invert= 476.00'



#### Hydrograph 0.007 - Inflow 0.01 cfs - Outflow 0.007 Inflow Area=0.471 ac 0.006 Avg. Flow Depth=0.03' 0.006 0.005 Max Vel=0.95 fps 0.005 12.0" 0.004 low (cfs) **Round Pipe** 0.004 0.003 n=0.012 0.003 L=50.0' 0.002 S=0.0100 '/' 0.002 Capacity=3.86 cfs 0.001 0.001 0.000-0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Ó Time (hours)

### Reach OS 1.1P: Outlet Pipe

### **Post-Development**

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### Stage-Area-Storage for Reach OS 1.1P: Outlet Pipe

Post-Development	NY-Waccabuc 24-hr S0P	1-yr Rainfall=2.82"
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### Summary for Pond 1.1P: Pond

Inflow Area =	0.471 ac,	43.41% Impervious,	Inflow Depth =	0.99" for	1-yr event
Inflow =	0.59 cfs @	12.04 hrs, Volume	e= 1,700 cf		
Outflow =	0.01 cfs @	24.06 hrs, Volume	e 1,243 cf,	Atten= 99%,	, Lag= 721.0 min
Primary =	0.01 cfs @	24.06 hrs, Volume	e= 1,243 cf		

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Starting Elev= 482.00' Surf.Area= 2,200 sf Storage= 2,420 cf Peak Elev= 482.58' @ 24.06 hrs Surf.Area= 2,664 sf Storage= 3,830 cf (1,410 cf above start)

Plug-Flow detention time= (not calculated: initial storage exceeds outflow) Center-of-Mass det. time= 1,497.1 min (2,363.2 - 866.1)

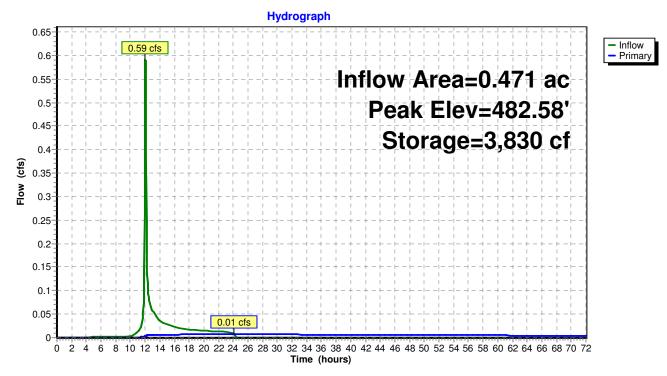
Volume	Inv	ert Avail.Sto	orage Storage	Description	
#1	477.	00' 8,4	20 cf Custon	n Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
477.0	00	20	0	0	
479.0	00	200	220	220	
481.0	00	600	800	1,020	
482.0	00	2,200	1,400	2,420	
484.0	00	3,800	6,000	8,420	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	482.00'	0.6" Vert. Or	ifice/Grate C=	0.600
#2 Primary 482.70' <b>1.5' long x 0.5' breadth Broad-Crested Rectangular Weir X 4.00</b> Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32					
Primary OutFlow Max=0.01 cfs @ 24.06 hrs HW=482.58' TW=476.53' (Dynamic Tailwater)					

**1=Orifice/Grate** (Orifice Controls 0.01 cfs @ 3.59 fps)

2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

# Post-DevelopmentNY-Waccabuc 24-hr SOP 1-yr Rainfall=2.82"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 13

Pond 1.1P: Pond



### **Post-Development**

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		-	-		
Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
	20		482.20		
477.00		0		2,360	2,876
477.10	29	2	482.30	2,440	3,116
477.20	38	6	482.40	2,520	3,364
477.30	47	10	482.50	2,600	3,620
477.40	56	15	482.60	2,680	3,884
477.50	65	21	482.70	2,760	4,156
477.60	74	28	482.80	2,840	4,436
477.70	83	36	482.90	2,920	4,724
477.80	92	45	483.00	3,000	5,020
477.90	101	54	483.10	3,080	5,324
478.00	110	65	483.20	3,160	5,636
478.10	119	76	483.30	3,240	5,956
478.20	128	89	483.40	3,320	6,284
478.30	137	102	483.50	3,400	6,620
478.40	146	116	483.60	3,480	6,964
478.50	155	131	483.70	3,560	7,316
478.60	164	147	483.80	3,640	7,676
478.70	173	164	483.90	3,720	8,044
478.80	182	182	484.00	3,800	8,420
478.90	191	200			
479.00	200	220			
479.10	220	241			
479.20	240	264			
479.30	260	289			
479.40	280	316			
479.50	300	345			
479.60	320	376			
479.70	340	409			
479.80	360	444			
479.90	380	481			
480.00	400	520			
480.10	420	561			
480.20	440	604			
480.30	460	649			
480.40	480	696			
480.50	500	745			
480.60	520	796			
480.70	540	849			
480.80	560	904			
480.90	580	961			
481.00	600	1,020			
481.10	760	1,088			
481.20	920	1,172			
481.30	1,080	1,272			
481.40	1,240	1,388			
481.50	1,400	1,520			
481.60	1,560	1,668			
481.70	1,720	1,832			
481.80	1,880	2,012			
481.90	2,040	2,208			
482.00	2,200	2,420			
482.10	2,280	2,644			
		l			

### Stage-Area-Storage for Pond 1.1P: Pond

Post-Development	NY-Waccabuc 24-hr S0P 10-	-yr Rainfall=5.07"
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# Summary for Subcatchment 1.0S:

Runoff = 12.40 cfs @ 12.38 hrs, Volume= 73,546 cf, Depth= 1.93"

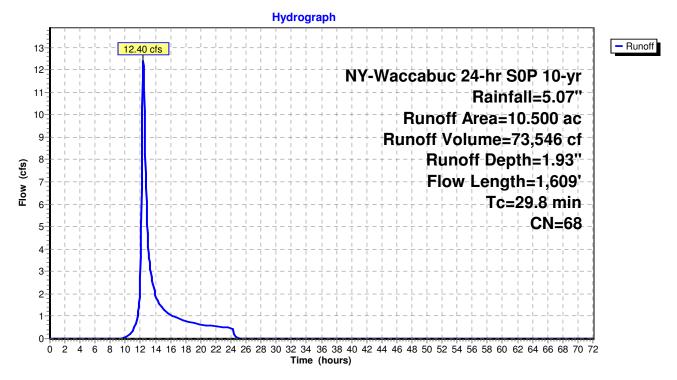
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 10-yr Rainfall=5.07"

Area	(ac) C	N Dese	cription				
7	.800 7	70 Woo	ds, Good,	HSG C			
* 0.	.220 9		ed parking				
0	.310 9	96 Grav	vel surface	, HSG C			
2	.120 5	55 Woo	ds, Good,	HSG B			
0	.050 6	51 >759	% Grass c	over, Good	, HSG B		
10	10.500 68 Weighted Average						
10	.280	97.9	0% Pervio	us Area			
0.	.220	2.10	% Impervi	ous Area			
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
14.4	100	0.0500	0.12		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.40"		
6.9	550	0.0700	1.32		Shallow Concentrated Flow,		
					Woodland Kv= 5.0 fps		
0.1	23	0.0400	4.06		Shallow Concentrated Flow,		
					Paved Kv= 20.3 fps		
0.5	51	0.1000	1.58		Shallow Concentrated Flow,		
			. =-		Woodland Kv= 5.0 fps		
5.4	565	0.1200	1.73		Shallow Concentrated Flow,		
			40.04		Woodland Kv= 5.0 fps		
0.0	32	0.1340	16.61	13.04			
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'		
		0 1 1 0 0	04 70	00 74	n= 0.013 Corrugated PE, smooth interior		
0.2	268	0.1460	21.79	26.74			
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
			0.45		n= 0.012		
2.3	20	0.2000	0.15		Sheet Flow,		
					Woods: Light underbrush n= 0.400 P2= 3.40"		
29.8	1,609	Total					

29.8 1,609 Total

# Post-DevelopmentNY-Waccabuc 24-hr S0P 10-yrRainfall=5.07"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 16

Subcatchment 1.0S:



#### Summary for Subcatchment 1.1 S: (New)

Runoff = 1.38 cfs @ 12.04 hrs, Volume= 4,171 cf, Depth= 2.59"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 10-yr Rainfall=5.07"

Area (sf) 7,700 11,600 19,300 11,600 7,700	61 >75% Grass cover, Good, HSG B 76 Weighted Average 60.10% Pervious Area
Tc Lengtl (min) (feet	) (ft/ft) (ft/sec) (cfs)
5.0	Direct Entry,
	Subcatchment 1.1 S: (New)
	Hydrograph
Elow (cts)	1.38 cfs       - Runoff         NY-Waccabuc 24-hr S0P 10-yr       Rainfall=5.07"         Runoff Area=19,300 sf       Runoff Volume=4,171 cf         Runoff Depth=2.59"       Tc=5.0 min         CN=76       CN=76

0 2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72 Time (hours)

Post-Development	NY-Waccabuc 24-hr S0P 10	-yr Rainfall=5.07"
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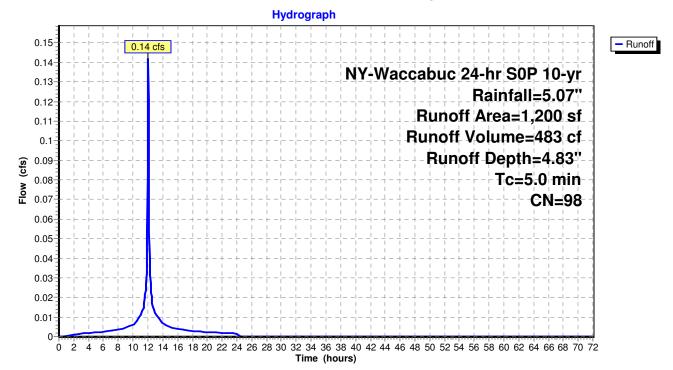
#### Summary for Subcatchment 1.1S: (Redevelopment)

Runoff = 0.14 cfs @ 12.03 hrs, Volume= 483 cf, Depth= 4.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 10-yr Rainfall=5.07"

A	rea (sf)	CN E	Description		
	1,200	98 F	aved park	ing, HSG B	3
	1,200	1	00.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

#### Subcatchment 1.1S: (Redevelopment)

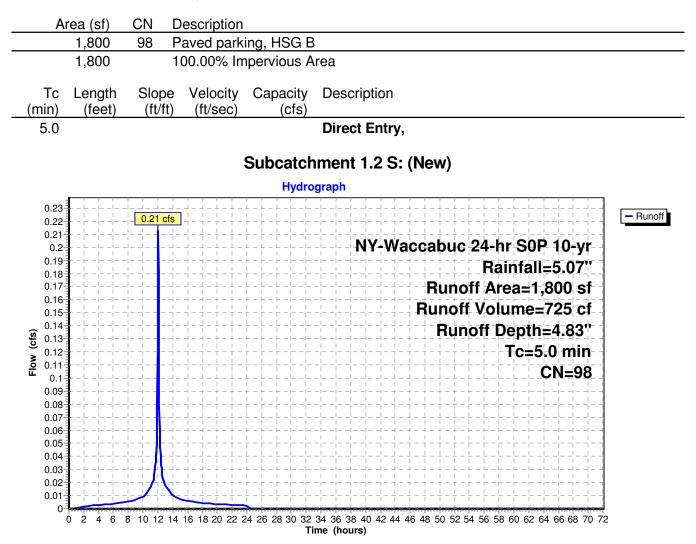


Post-Development	NY-Waccabuc 24-hr S0P 10	-yr Rainfall=5.07"
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#### Summary for Subcatchment 1.2 S: (New)

Runoff = 0.21 cfs @ 12.03 hrs, Volume= 725 cf, Depth= 4.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 10-yr Rainfall=5.07"



## Post-DevelopmentNY-Waccabuc 24-hr SOP 10-yrRainfall=5.07"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15s/n 02171© 2015 HydroCAD Software Solutions LLCPage 20

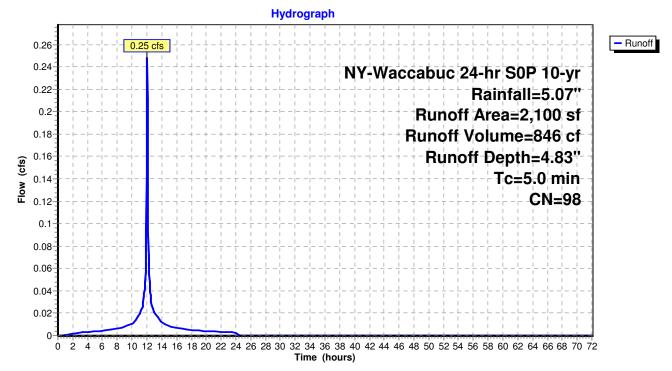
#### Summary for Subcatchment 1.2S: (Redevelopment)

Runoff = 0.25 cfs @ 12.03 hrs, Volume= 846 cf, Depth= 4.83"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 10-yr Rainfall=5.07"

A	rea (sf)	CN [	Description		
	2,100	98 F	Paved park	ing, HSG B	3
	2,100	1	00.00% In	npervious A	vrea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

#### Subcatchment 1.2S: (Redevelopment)



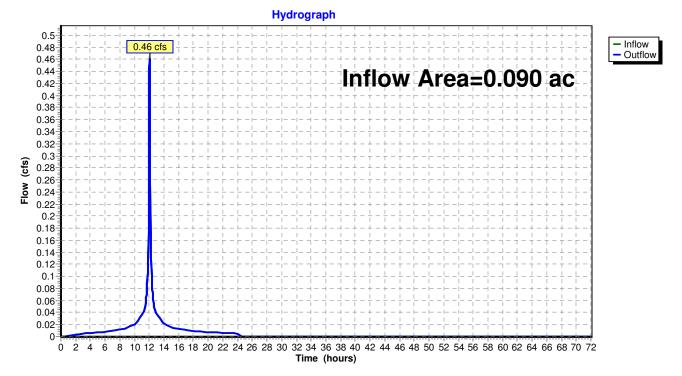
Post-Development	NY-Waccabuc 24-hr S0P 10	-yr Rainfall=5.07"
Prepared by Insite Engineering, Surveying & La	andscape Architecture, P.C.	Printed 2/3/2023
HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD S	Software Solutions LLC	Page 21

### Summary for Reach 1.2P: Cistern

Inflow Area	a =	0.090 ac,10	00.00% Impervious, Inf	low Depth = $4.83$ "	for 10-yr event
Inflow	=	0.46 cfs @	12.03 hrs, Volume=	1,571 cf	
Outflow	=	0.46 cfs @	12.03 hrs, Volume=	1,571 cf, Atten=	0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

#### Reach 1.2P: Cistern



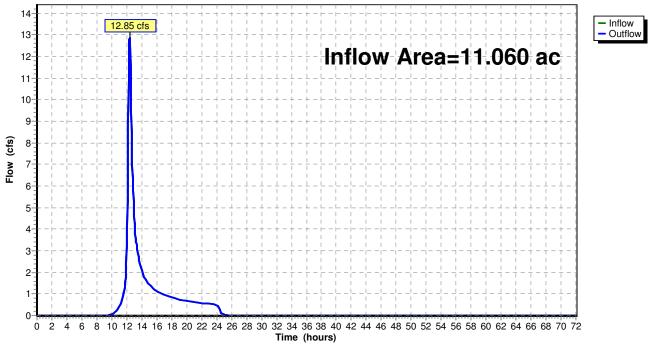
Post-Development	NY-Waccabuc 24-hr S0P 10	-yr Rainfall=5.07"
Prepared by Insite Engineering, Surveying & La	Indscape Architecture, P.C.	Printed 2/3/2023
HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD So	oftware Solutions LLC	Page 22

#### Summary for Reach DL 1: Design Line 1

Inflow Area	a =	11.060 ac,	4.65% Impervious, Inf	flow Depth > 1	.97" for 10-yr event
Inflow	=	12.85 cfs @	12.38 hrs, Volume=	79,112 cf	
Outflow	=	12.85 cfs @	12.38 hrs, Volume=	79,112 cf, Att	ten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Reach DL 1: Design Line 1 Hydrograph



Post-DevelopmentNY-Waccabuc 24-hr SOP 10-yrRainfall=5.07"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 23

#### Summary for Reach OS 1.1P: Outlet Pipe

 Inflow Area =
 0.471 ac, 43.41% Impervious, Inflow Depth > 2.34" for 10-yr event

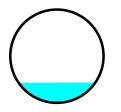
 Inflow =
 0.40 cfs @ 12.30 hrs, Volume=
 3,996 cf

 Outflow =
 0.40 cfs @ 12.31 hrs, Volume=
 3,996 cf, Atten= 1%, Lag= 0.4 min

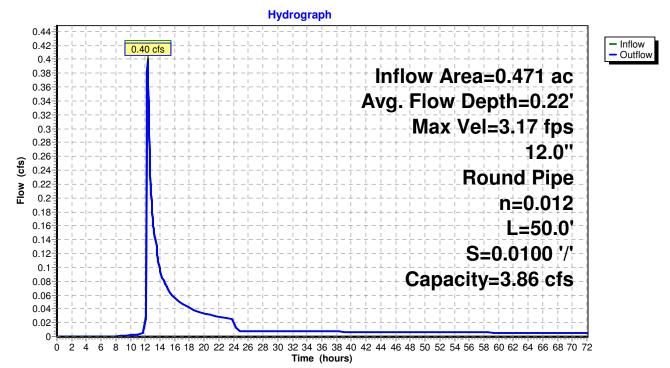
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 3.17 fps, Min. Travel Time= 0.3 min Avg. Velocity = 1.02 fps, Avg. Travel Time= 0.8 min

Peak Storage= 6 cf @ 12.31 hrs Average Depth at Peak Storage= 0.22' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.86 cfs

12.0" Round Pipe n= 0.012 Length= 50.0' Slope= 0.0100 '/' Inlet Invert= 476.50', Outlet Invert= 476.00'



#### Reach OS 1.1P: Outlet Pipe



#### **Post-Development**

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ElevationEnd-AreaStorageElevationEnd-AreaStorage $\frac{(feet)}{(sq,ft)}$ $(cubic-feet)$ $477.02$ $0.4$ $21$ $476.51$ $0.0$ $0$ $477.03$ $0.4$ $21$ $476.52$ $0.0$ $0$ $477.03$ $0.4$ $21$ $476.53$ $0.0$ $0$ $477.05$ $0.4$ $22$ $476.54$ $0.0$ $1$ $477.05$ $0.4$ $22$ $476.55$ $0.0$ $1$ $477.06$ $0.5$ $233$ $476.56$ $0.0$ $1$ $477.09$ $0.5$ $24$ $476.57$ $0.0$ $1$ $477.09$ $0.5$ $24$ $476.58$ $0.0$ $2$ $477.11$ $0.5$ $25$ $476.69$ $0.0$ $2$ $477.12$ $0.5$ $26$ $476.61$ $0.0$ $2$ $477.14$ $0.5$ $27$ $476.62$ $0.1$ $3$ $477.16$ $0.5$ $27$ $476.63$ $0.1$ $3$ $477.16$ $0.5$ $27$ $476.64$ $0.1$ $3$ $477.16$ $0.5$ $27$ $476.65$ $0.1$ $4$ $477.18$ $0.6$ $28$ $476.67$ $0.1$ $4$ $477.20$ $0.6$ $29$ $476.69$ $0.1$ $5$ $477.20$ $0.6$ $29$ $476.69$ $0.1$ $5$ $477.20$ $0.6$ $32$ $476.70$ $0.1$ $6$ $477.20$ $0.6$ $32$ $476.70$ $0.1$ $6$ $477.20$ $0.6$ $32$ <						0																																																																																																																																																																																													
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#### Stage-Area-Storage for Reach OS 1.1P: Outlet Pipe

Post-Development	NY-Waccabuc 24-hr S0P 10	yr Rainfall=5.07"
Prepared by Insite Engineering, Surveying & Lan	dscape Architecture, P.C.	Printed 2/3/2023
HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Sof	tware Solutions LLC	Page 25

#### Summary for Pond 1.1P: Pond

Inflow Area =	0.471 ac, 43.41% Impervious, Inflow Depth = 2.72" for 10-yr event	
Inflow =	1.52 cfs @ 12.03 hrs, Volume= 4,654 cf	
Outflow =	0.40 cfs @ 12.30 hrs, Volume= 3,996 cf, Atten= 74%, Lag= 16.1 mir	۱
Primary =	0.40 cfs @ 12.30 hrs, Volume= 3,996 cf	

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Starting Elev= 482.00' Surf.Area= 2,200 sf Storage= 2,420 cf Peak Elev= 482.78' @ 12.30 hrs Surf.Area= 2,825 sf Storage= 4,384 cf (1,964 cf above start)

Plug-Flow detention time= 1,645.3 min calculated for 1,576 cf (34% of inflow) Center-of-Mass det. time= 603.4 min (1,439.0 - 835.6)

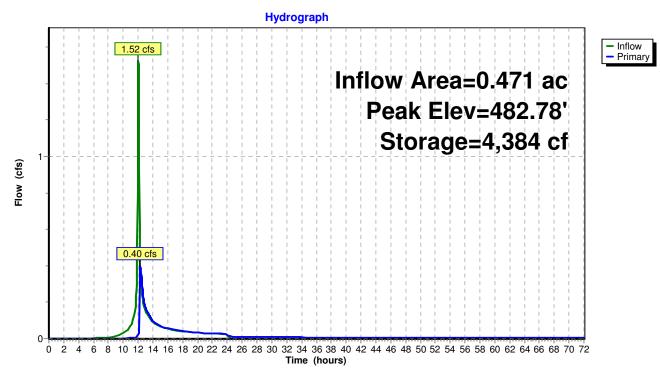
Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	477.0	0' 8,42	20 cf Custom	Stage Data (Pr	ismatic) Listed below (Recalc)
Elevatio (feet	t)	Surf.Area (sq-ft) 20	Inc.Store (cubic-feet) 0	Cum.Store (cubic-feet) 0	
479.0	-	200	220	220	
481.0	0	600	800	1,020	
482.0	0	2,200	1,400	2,420	
484.0	0	3,800	6,000	8,420	
Device	Routing	Invert	Outlet Device	S	
#1 #2	Primary Primary	482.00' 482.70'	<b>1.5' long x 0</b> Head (feet) 0	fice/Grate C= .5' breadth Broa 0.20 0.40 0.60 h) 2.80 2.92 3.	d-Crested Rectangular Weir X 4.00 0.80 1.00
Primary	OutFlow	Max=0.40 cfs (	@ 12.30 hrs H	W=482.78' TW=	=476.72' (Dynamic Tailwater)

**1=Orifice/Grate** (Orifice Controls 0.01 cfs @ 4.19 fps)

2=Broad-Crested Rectangular Weir (Weir Controls 0.39 cfs @ 0.80 fps)

#### **Post-Development**

Pond 1.1P: Pond



### **Post-Development**

<u>e 27</u>

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### Stage-Area-Storage for Pond 1.1P: Pond

Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
477.00	20	0	482.20	2,360	2,876
477.10	29	2	482.30	2,440	3,116
477.20	38	6	482.40	2,520	3,364
477.30	47	10	482.50	2,600	3,620
477.40	56	15	482.60	2,680	3,884
477.50	65	21	482.70	2,760	4,156
477.60	74	28	482.80	2,840	4,436
477.70	83	36	482.90	2,840 2,920	
477.80	92	45			4,724
			483.00	3,000	5,020
477.90	101	54	483.10	3,080	5,324
478.00	110	65	483.20	3,160	5,636
478.10	119	76	483.30	3,240	5,956
478.20	128	89	483.40	3,320	6,284
478.30	137	102	483.50	3,400	6,620
478.40	146	116	483.60	3,480	6,964
478.50	155	131	483.70	3,560	7,316
478.60	164	147	483.80	3,640	7,676
478.70	173	164	483.90	3,720	8,044
478.80	182	182	484.00	3,800	8,420
478.90	191	200			
479.00	200	220			
479.10	220	241			
479.20	240	264			
479.30	260	289			
479.40	280	316			
479.50	300	345			
479.60	320	376			
479.70	340	409			
479.80	360	444			
479.90	380	481			
480.00	400	520			
480.10	420	561			
480.20	440	604			
480.30	460	649			
480.40	480	696			
480.50	500	745			
480.60 480.70	520	796 849			
	540	• • •			
480.80	560	904			
480.90	580	961			
481.00	600	1,020			
481.10	760	1,088			
481.20	920	1,172			
481.30	1,080	1,272			
481.40	1,240	1,388			
481.50	1,400	1,520			
481.60	1,560	1,668			
481.70	1,720	1,832			
481.80	1,880	2,012			
481.90	2,040	2,208			
482.00	2,200	2,420			
482.10	2,280	2,644			

#### Summary for Subcatchment 1.0S:

Runoff = 31.23 cfs @ 12.37 hrs, Volume= 194,581 cf, Depth= 5.11"

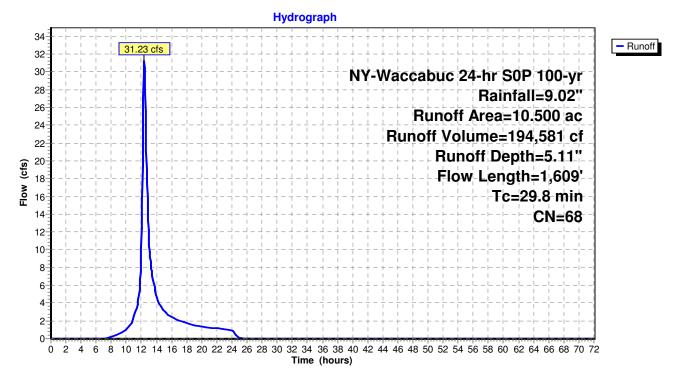
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 100-yr Rainfall=9.02"

7.800 70 Woods, Good, HSG C	
* 0.220 98 Paved parking	
0.310 96 Gravel surface, HSG C	
2.120 55 Woods, Good, HSG B	
0.050 61 >75% Grass cover, Good,	HSG B
10.500 68 Weighted Average	
10.280 97.90% Pervious Area	
0.220 2.10% Impervious Area	
	Description
(min) (feet) (ft/ft) (ft/sec) (cfs)	
14.4 100 0.0500 0.12	Sheet Flow,
	Woods: Light underbrush n= 0.400 P2= 3.40"
6.9 550 0.0700 1.32	Shallow Concentrated Flow,
	Woodland Kv= 5.0 fps
0.1 23 0.0400 4.06	Shallow Concentrated Flow,
	Paved Kv= 20.3 fps
0.5 51 0.1000 1.58	Shallow Concentrated Flow,
	Woodland Kv= 5.0 fps
5.4 565 0.1200 1.73	Shallow Concentrated Flow,
0.0 00 0.1040 10.01 10.04	Woodland Kv= 5.0 fps
0.0 32 0.1340 16.61 13.04	Pipe Channel,
	12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
	n= 0.013 Corrugated PE, smooth interior
0.2 200 0.1400 21.79 20.74	<b>Pipe Channel,</b> 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
	n = 0.012
2.3 20 0.2000 0.15	Sheet Flow,
2.3 20 0.2000 0.15	Woods: Light underbrush n= 0.400 P2= 3.40"
29.8 1.609 Total	

29.8 1,609 Total

# Post-DevelopmentNY-Waccabuc 24-hr S0P 100-yrRainfall=9.02"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 29

Subcatchment 1.0S:



#### Summary for Subcatchment 1.1 S: (New)

Runoff = 2.76 cfs @ 12.03 hrs, Volume= 9,802 cf, Depth= 6.09"

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Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 100-yr Rainfall=9.02"

Area (s	f) CN	Description	1		
7,70		Paved park			
11,60				ood, HSG B	
19,30		Weighted A			
11,60		60.10% Pe			
7,70	0	39.90% lm	pervious Are	ea	
Tc Lenç			Capacity	Description	
(min) (fe	et) (ft/	t) (ft/sec)	(cfs)		
5.0				Direct Entry,	
			Subcatch	nment 1.1 S: (New)	
Hydrograph					
·			Hydro	graph	
3			<b>Hydro</b>	<b>graph</b>	
3			Hydro		
3			Hydro	NY-Waccabuc 24-hr S0P 100-	yr
3		S	<b>Hydro</b>		yr
3		S	<b>Hydro</b>	NY-Waccabuc 24-hr S0P 100-	yr 2"
3		S	Hydro	NY-Waccabuc 24-hr S0P 100- Rainfall=9.0 Runoff Area=19,300	yr 2'' sf
3			Hydro	NY-Waccabuc 24-hr S0P 100- Rainfall=9.0 Runoff Area=19,300 Runoff Volume=9,802	yr 2" sf cf-
			Hydro	NY-Waccabuc 24-hr S0P 100- Rainfall=9.0 Runoff Area=19,300 Runoff Volume=9,802 Runoff Depth=6.0	yr 2" sf cf- 9"
3C		I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I       I	Hydro	NY-Waccabuc 24-hr S0P 100- Rainfall=9.0 Runoff Area=19,300 Runoff Volume=9,802	yr 2" sf cf- 9"

2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 50 52 54 56 58 60 62 64 66 68 70 72

Time (hours)

## Post-DevelopmentNY-Waccabuc 24-hr S0P 100-yrRainfall=9.02"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 31

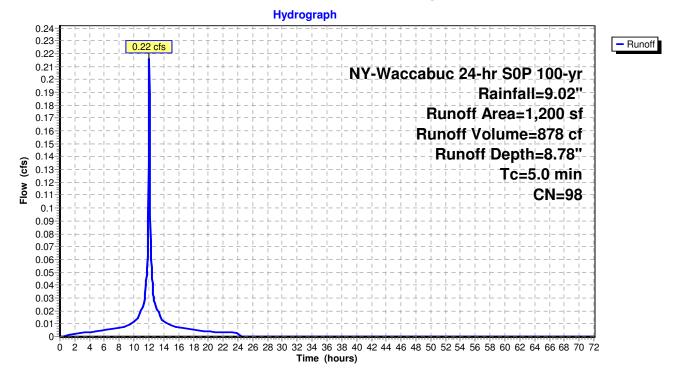
#### Summary for Subcatchment 1.1S: (Redevelopment)

Runoff = 0.22 cfs @ 12.03 hrs, Volume= 878 cf, Depth= 8.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 100-yr Rainfall=9.02"

A	rea (sf)	CN	Description		
	1,200	98	Paved park	ing, HSG B	3
	1,200		100.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
5.0					Direct Entry,

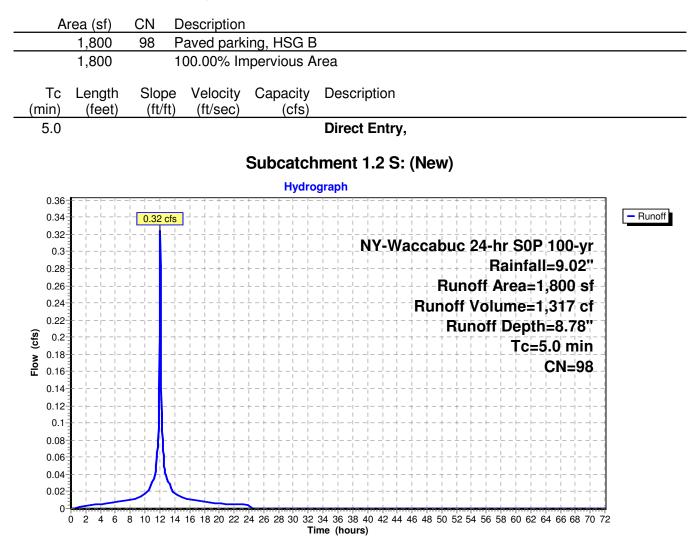
#### Subcatchment 1.1S: (Redevelopment)



#### Summary for Subcatchment 1.2 S: (New)

Runoff = 0.32 cfs @ 12.03 hrs, Volume= 1,317 cf, Depth= 8.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 100-yr Rainfall=9.02"



## Post-DevelopmentNY-Waccabuc 24-hr S0P 100-yrRainfall=9.02"Prepared by Insite Engineering, Surveying & Landscape Architecture, P.C.Printed 2/3/2023HydroCAD® 10.00-15 s/n 02171 © 2015 HydroCAD Software Solutions LLCPage 33

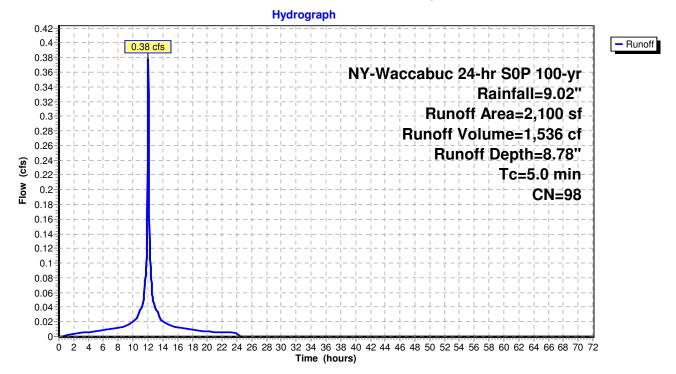
#### Summary for Subcatchment 1.2S: (Redevelopment)

Runoff = 0.38 cfs @ 12.03 hrs, Volume= 1,536 cf, Depth= 8.78"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs NY-Waccabuc 24-hr S0P 100-yr Rainfall=9.02"

A	rea (sf)	CN I	Description		
	2,100	98 I	Paved park	ing, HSG B	3
	2,100		100.00% In	npervious A	Area
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.0					Direct Entry,

#### Subcatchment 1.2S: (Redevelopment)

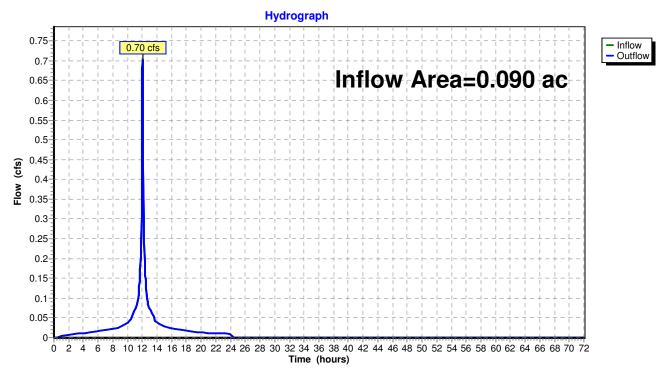


Post-Development	NY-Waccabuc 24-hr S0P 100-	yr Rainfall=9.02"
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### Summary for Reach 1.2P: Cistern

Inflow Area	a =	0.090 ac,10	00.00% Impervious, Infl	ow Depth = 8.78"	for 100-yr event
Inflow	=	0.70 cfs @	12.03 hrs, Volume=	2,853 cf	
Outflow	=	0.70 cfs @	12.03 hrs, Volume=	2,853 cf, Atten=	0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs



#### Reach 1.2P: Cistern

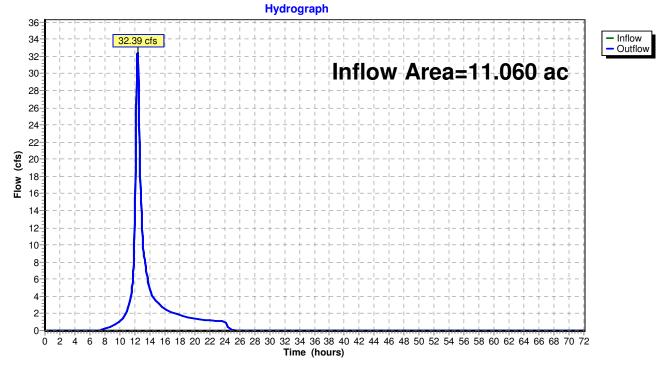
Post-Development	NY-Waccabuc 24-hr S0P 10	00-yr Rainfall=9.02"
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### Summary for Reach DL 1: Design Line 1

Inflow Area	a =	11.060 ac,	4.65% Impervious, Inflow Depth > 5.17" for 100-yr event
Inflow	=	32.39 cfs @	12.37 hrs, Volume= 207,453 cf
Outflow	=	32.39 cfs @	12.37 hrs, Volume= 207,453 cf, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs

## Reach DL 1: Design Line 1



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#### Summary for Reach OS 1.1P: Outlet Pipe

 Inflow Area =
 0.471 ac, 43.41% Impervious, Inflow Depth > 5.86" for 100-yr event

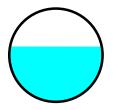
 Inflow =
 2.54 cfs @ 12.09 hrs, Volume=
 10,018 cf

 Outflow =
 2.55 cfs @ 12.10 hrs, Volume=
 10,018 cf, Atten= 0%, Lag= 0.1 min

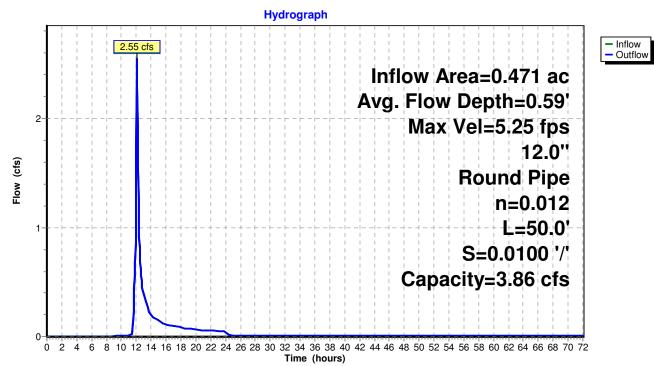
Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Max. Velocity= 5.25 fps, Min. Travel Time= 0.2 min Avg. Velocity = 1.12 fps, Avg. Travel Time= 0.7 min

Peak Storage= 24 cf @ 12.10 hrs Average Depth at Peak Storage= 0.59' Bank-Full Depth= 1.00' Flow Area= 0.8 sf, Capacity= 3.86 cfs

12.0" Round Pipe n= 0.012 Length= 50.0' Slope= 0.0100 '/' Inlet Invert= 476.50', Outlet Invert= 476.00'



#### Reach OS 1.1P: Outlet Pipe



#### **Post-Development**

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#### Elevation End-Area Storage Elevation End-Area Storage (cubic-feet) (feet) (cubic-feet) (feet) (sq-ft) (sq-ft) 476.50 477.02 0.4 0.0 0 21 476.51 0.0 0 477.03 0.4 21 0 22 476.52 0.0 0.4 477.04 0 22 476.53 0.0 0.4 477.05 23 476.54 0.0 1 477.06 0.5 476.55 1 0.5 23 0.0 477.07 476.56 1 477.08 0.5 24 0.0 476.57 1 477.09 0.5 24 0.0 25 476.58 0.0 1 477.10 0.5 2 2 25 476.59 0.0 477.11 0.5 476.60 0.0 477.12 0.5 26 2 0.5 26 476.61 0.0 477.13 3 0.5 27 476.62 0.1 477.14 3 27 0.5 476.63 0.1 477.15 3 27 0.5 476.64 0.1 477.16 476.65 0.1 4 477.17 0.6 28 4 28 476.66 0.1 477.18 0.6 4 476.67 0.1 477.19 0.6 29 5 29 476.68 0.1 477.20 0.6 5 30 476.69 0.1 477.21 0.6 6 30 476.70 0.1 477.22 0.6 476.71 0.1 6 477.23 0.6 31 6 476.72 0.1 477.24 0.6 31 476.73 7 0.6 32 0.1 477.25 7 476.74 0.6 32 0.1 477.26 8 0.2 32 476.75 477.27 0.6 476.76 0.2 8 477.28 0.7 33 476.77 0.2 9 477.29 0.7 33 9 476.78 0.2 477.30 0.7 34 34 476.79 0.2 9 477.31 0.7 0.2 0.7 34 476.80 10 477.32 35 0.2 0.7 476.81 10 477.33 476.82 0.2 11 477.34 0.7 35 0.7 36 476.83 0.2 11 477.35 476.84 0.2 0.7 36 12 477.36 0.2 12 0.7 36 476.85 477.37 0.7 37 476.86 0.3 13 477.38 476.87 0.3 13 477.39 0.7 37 0.7 37 476.88 0.3 14 477.40 476.89 0.3 14 477.41 0.8 38 476.90 0.3 15 477.42 0.8 38 476.91 0.3 15 477.43 0.8 38 38 476.92 0.3 16 477.44 0.8 476.93 0.3 16 477.45 0.8 39 476.94 0.3 17 477.46 0.8 39 476.95 0.3 17 477.47 0.8 39 476.96 0.4 18 477.48 0.8 39 476.97 0.4 18 477.49 0.8 39 476.98 0.4 19 477.50 0.8 39 476.99 0.4 19 477.00 0.4 20 20 477.01 0.4

#### Stage-Area-Storage for Reach OS 1.1P: Outlet Pipe

Post-Development	NY-Waccabuc 24-hr S0P 100	yr Rainfall=9.02"
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#### Summary for Pond 1.1P: Pond

Inflow Area =	0.471 ac, 43.41% Impervious, Inflow Depth = 6.25" for 100-yr event
Inflow =	2.97 cfs @ 12.03 hrs, Volume= 10,679 cf
Outflow =	2.54 cfs @ 12.09 hrs, Volume= 10,018 cf, Atten= 14%, Lag= 3.7 min
Primary =	2.54 cfs @ 12.09 hrs, Volume= 10,018 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-72.00 hrs, dt= 0.05 hrs Starting Elev= 482.00' Surf.Area= 2,200 sf Storage= 2,420 cf Peak Elev= 482.98' @ 12.09 hrs Surf.Area= 2,984 sf Storage= 4,960 cf (2,540 cf above start)

Plug-Flow detention time= 478.0 min calculated for 7,598 cf (71% of inflow) Center-of-Mass det. time= 259.9 min (1,069.2 - 809.3)

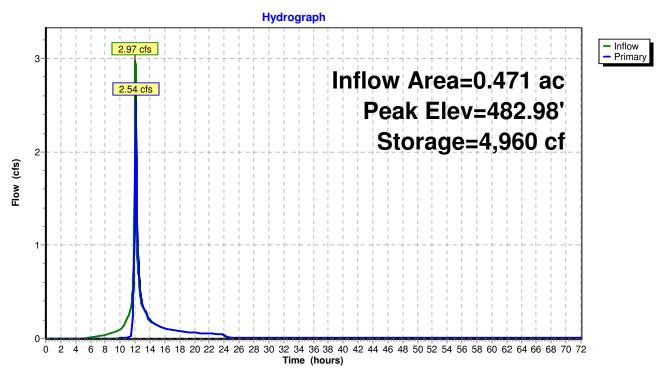
Volume	Inve	ert Avail.Sto	rage Storage	Description	
#1	477.0	00' 8,42	20 cf Custom	n Stage Data (Pri	ismatic) Listed below (Recalc)
Elevatio (feet 477.0 479.0 481.0 482.0 484.0	t) 0 0 0 0	Surf.Area (sq-ft) 20 200 600 2,200 3,800	Inc.Store (cubic-feet) 0 220 800 1,400 6,000	Cum.Store (cubic-feet) 0 220 1,020 2,420 8,420	
Device	Routing	Invert	Outlet Device	es	
#1 #2	Primary Primary	482.00' 482.70'	<b>1.5' long x 0</b> Head (feet) (	ifice/Grate C= .5' breadth Broa 0.20 0.40 0.60 h) 2.80 2.92 3.	d-Crested Rectangular Weir X 4.00 0.80 1.00
Primary	OutFlow	Max=2.52 cfs (	@ 12.09 hrs H	W=482.98' TW=	-477.09' (Dynamic Tailwater)

**1=Orifice/Grate** (Orifice Controls 0.01 cfs @ 4.70 fps)

2=Broad-Crested Rectangular Weir (Weir Controls 2.51 cfs @ 1.50 fps)

#### **Post-Development**

Pond 1.1P: Pond



#### **Post-Development**

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Elevation	Surface	Storage	Elevation	Surface	Storage
(feet)	(sq-ft)	(cubic-feet)	(feet)	(sq-ft)	(cubic-feet)
477.00	20	0	482.20	2,360	2,876
477.10	29	2	482.30	2,440	3,116
477.20	38	6	482.40	2,520	3,364
477.30	47	10	482.50	2,600	3,620
477.40	56	15	482.60	2,680	3,884
477.50	65	21	482.70	2,760	4,156
477.60	74	28	482.80	2,840	4,436
477.70	83	36	482.90	2,920	4,724
477.80	92	45	483.00	3,000	5,020
477.90	101	54	483.10		
478.00	110	65	483.20	3,080	5,324
				3,160	5,636
478.10	119	76	483.30	3,240	5,956
478.20	128	89	483.40	3,320	6,284
478.30	137	102	483.50	3,400	6,620
478.40	146	116	483.60	3,480	6,964
478.50	155	131	483.70	3,560	7,316
478.60	164	147	483.80	3,640	7,676
478.70	173	164	483.90	3,720	8,044
478.80	182	182	484.00	3,800	8,420
478.90	191	200			
479.00	200	220			
479.10	220	241			
479.20	240	264			
479.30	260	289			
479.40	280	316			
479.50	300	345			
479.60	320	376			
479.70	340	409			
479.80	360	444			
479.90	380	481			
480.00	400	520			
480.10	420	561			
480.20	440	604			
480.30	460	649			
480.40	480	696			
480.50	500	745			
480.60	520	796			
480.70	540	849			
480.80	560	904			
480.90	580	961			
481.00	600	1,020			
481.10	760	1,088			
481.20	920	1,172			
	1,080	1,272			
481.30 481.40					
	1,240	1,388			
481.50	1,400	1,520			
481.60	1,560	1,668			
481.70	1,720	1,832			
481.80	1,880	2,012			
481.90	2,040	2,208			
482.00	2,200	2,420			
482.10	2,280	2,644			
			I		

### APPENDIX D

NYSDEC SPDES for Construction Activities Construction Site Log Book

## APPENDIX F CONSTRUCTION SITE INSPECTION AND MAINTENANCE LOG BOOK

## STATE POLLUTANT DISCHARGE ELIMINATION SYSTEM FOR CONSTRUCTION ACTIVITIES

## SAMPLE CONSTRUCTION SITE LOG BOOK

### Table of Contents

- I. Pre-Construction Meeting Documents
  - a. Preamble to Site Assessment and Inspections
  - b. Pre-Construction Site Assessment Checklist

## **II.** Construction Duration Inspections

- a. Directions
- b. Modification to the SWPPP

#### I. PRE-CONSTRUCTION MEETING DOCUMENTS

Project Name	
Permit No.	Date of Authorization
Name of Operator	
Prime Contractor	

#### a. Preamble to Site Assessment and Inspections

The Following Information To Be Read By All Person's Involved in The Construction of Stormwater Related Activities:

The Operator agrees to have a qualified inspector¹ conduct an assessment of the site prior to the commencement of construction² and certify in this inspection report that the appropriate erosion and sediment controls described in the SWPPP have been adequately installed or implemented to ensure overall preparedness of the site for the commencement of construction.

Prior to the commencement of construction, the Operator shall certify in this site logbook that the SWPPP has been prepared in accordance with the State's standards and meets all Federal, State and local erosion and sediment control requirements. A preconstruction meeting should be held to review all of the SWPPP requirements with construction personnel.

When construction starts, site inspections shall be conducted by the qualified inspector at least every 7 calendar days. The Operator shall maintain a record of all inspection reports in this site logbook. The site logbook shall be maintained on site and be made available to the permitting authorities upon request.

Prior to filing the Notice of Termination or the end of permit term, the Operator shall have a qualified inspector perform a final site inspection. The qualified inspector shall certify that the site has undergone final stabilization³ using either vegetative or structural stabilization methods and that all temporary erosion and sediment controls (such as silt fencing) not needed for long-term erosion control have been removed. In addition, the Operator must identify and certify that all permanent structures described in the SWPPP have been constructed and provide the owner(s) with an operation and maintenance plan that ensures the structure(s) continuously functions as designed.

1 Refer to "Qualified Inspector" inspection requirements in the current SPDES General Permit for Stormwater Discharges from Construction Activity for complete list of inspection requirements.

3 "Final stabilization" means that all soil-disturbing activities at the site have been completed and a uniform, perennial vegetative cover with a density of eighty (80) percent has been established or equivalent stabilization measures (such as the use of mulches or geotextiles) have been employed on all unpaved areas and areas not covered by permanent structures.

^{2 &}quot;Commencement of construction" means the initial removal of vegetation and disturbance of soils associated with clearing, grading or excavating activities or other construction activities.

#### b. Pre-construction Site Assessment Checklist (NOTE: Provide comments below as necessary)

1. Notice of Intent, SWPPP, and Contractors Certification:

#### Yes No NA

- [] [] Has a Notice of Intent been filed with the NYS Department of Conservation?
- [] [] Is the SWPPP on-site? Where?
- [] [] Is the Plan current? What is the latest revision date?_____
- [] [] Is a copy of the NOI (with brief description) onsite? Where?
- [] [] Have all contractors involved with stormwater related activities signed a contractor's certification?

#### 2. Resource Protection

#### Yes No NA

- [] [] Are construction limits clearly flagged or fenced?
- [] [] Important trees and associated rooting zones, on-site septic system absorption fields, existing vegetated areas suitable for filter strips, especially in perimeter areas, have been flagged for protection.
- [] [] Creek crossings installed prior to land-disturbing activity, including clearing and blasting.
- 3. Surface Water Protection

#### Yes No NA

- [] [] Clean stormwater runoff has been diverted from areas to be disturbed.
- [] [] Bodies of water located either on site or in the vicinity of the site have been identified and protected.
- [] [] Appropriate practices to protect on-site or downstream surface water are installed.
- [] [] Are clearing and grading operations divided into areas <5 acres?
- 4. Stabilized Construction Access

#### Yes No NA

- [] [] A temporary construction entrance to capture mud and debris from construction vehicles before they enter the public highway has been installed.
- [] [] Other access areas (entrances, construction routes, equipment parking areas) are stabilized immediately as work takes place with gravel or other cover.
- [] [] Sediment tracked onto public streets is removed or cleaned on a regular basis.
- 5. Sediment Controls

#### Yes No NA

- [] [] Silt fence material and installation comply with the standard drawing and specifications.
- [] [] [] Silt fences are installed at appropriate spacing intervals
- [] [] Sediment/detention basin was installed as first land disturbing activity.
- [] [] [] Sediment traps and barriers are installed.

#### 6. Pollution Prevention for Waste and Hazardous Materials

#### Yes No NA

- [] [] The Operator or designated representative has been assigned to implement the spill prevention avoidance and response plan.
- [] [] The plan is contained in the SWPPP on page _
- [] [] Appropriate materials to control spills are onsite. Where?

#### **II. CONSTRUCTION DURATION INSPECTIONS**

#### a. Directions:

#### Inspection Forms will be filled out during the entire construction phase of the project.

Required Elements:

- 1) On a site map, indicate the extent of all disturbed site areas and drainage pathways. Indicate site areas that are expected to undergo initial disturbance or significant site work within the next 14-day period;
- 2) Indicate on a site map all areas of the site that have undergone temporary or permanent stabilization;
- 3) Indicate all disturbed site areas that have not undergone active site work during the previous 14-day period;
- 4) Inspect all sediment control practices and record the approximate degree of sediment accumulation as a percentage of sediment storage volume (for example, 10 percent, 20 percent, 50 percent);
- 5) Inspect all erosion and sediment control practices and record all maintenance requirements such as verifying the integrity of barrier or diversion systems (earthen berms or silt fencing) and containment systems (sediment basins and sediment traps). Identify any evidence of rill or gully erosion occurring on slopes and any loss of stabilizing vegetation or seeding/mulching. Document any excessive deposition of sediment or ponding water along barrier or diversion systems. Record the depth of sediment within containment structures, any erosion near outlet and overflow structures, and verify the ability of rock filters around perforated riser pipes to pass water; and
- 6) Immediately report to the Operator any deficiencies that are identified with the implementation of the SWPPP.

#### SITE PLAN/SKETCH

 Inspector (print name)
 Date of Inspection

 Qualified Inspector (print name)
 Qualified Inspector Signature

The above signed acknowledges that, to the best of his/her knowledge, all information provided on the forms is accurate and complete.

#### CONSTRUCTION DURATION INSPECTIONS

#### **Maintaining Water Quality**

#### Yes No NA

- [] [] Is there an increase in turbidity causing a substantial visible contrast to natural conditions at the outfalls?
- [] [] Is there residue from oil and floating substances, visible oil film, or globules or grease at the outfalls?
- [] [] All disturbance is within the limits of the approved plans.
- [] [] Have receiving lake/bay, stream, and/or wetland been impacted by silt from project?

#### Housekeeping

1. General Site Conditions

#### Yes No NA

- [] [] [] Is construction site litter, debris and spoils appropriately managed?
- [] [] [] Are facilities and equipment necessary for implementation of erosion and sediment control in working order and/or properly maintained?
- [] [] [] Is construction impacting the adjacent property?
- [] [] [] Is dust adequately controlled?

#### 2. Temporary Stream Crossing

#### Yes No NA

- [] [] Maximum diameter pipes necessary to span creek without dredging are installed.
- [] [] Installed non-woven geotextile fabric beneath approaches.
- [] [] Is fill composed of aggregate (no earth or soil)?
- [] [] Rock on approaches is clean enough to remove mud from vehicles & prevent sediment from entering stream during high flow.
- 3. Stabilized Construction Access

#### Yes No NA

- [] [] Stone is clean enough to effectively remove mud from vehicles.
- [] [] [] Installed per standards and specifications?
- [] [] Does all traffic use the stabilized entrance to enter and leave site?
- [] [] [] Is adequate drainage provided to prevent ponding at entrance?

#### **Runoff Control Practices**

1. Excavation Dewatering

#### Yes No NA

- [] [] Upstream and downstream berms (sandbags, inflatable dams, etc.) are installed per plan.
- [] [] Clean water from upstream pool is being pumped to the downstream pool.
- [] [] Sediment laden water from work area is being discharged to a silt-trapping device.
- [] [] Constructed upstream berm with one-foot minimum freeboard.

#### **Runoff Control Practices (continued)**

#### 2. Flow Spreader

#### Yes No NA

- [] [] [] Installed per plan.
- [] [] Constructed on undisturbed soil, not on fill, receiving only clear, non-sediment laden flow.
- [] [] Flow sheets out of level spreader without erosion on downstream edge.

#### 3. Interceptor Dikes and Swales

#### Yes No NA

- [] [] [] Installed per plan with minimum side slopes 2H:1V or flatter.
- [] [] Stabilized by geotextile fabric, seed, or mulch with no erosion occurring.
- [] [] [] Sediment-laden runoff directed to sediment trapping structure

#### 4. Stone Check Dam

#### Yes No NA

- [] [] [] Is channel stable? (flow is not eroding soil underneath or around the structure).
- [] [] Check is in good condition (rocks in place and no permanent pools behind the structure).
- [] [] Has accumulated sediment been removed?.

#### 5. Rock Outlet Protection

#### Yes No NA

- [] [] [] Installed per plan.
- [] [] Installed concurrently with pipe installation.

#### Soil Stabilization

1. Topsoil and Spoil Stockpiles

#### Yes No NA

- [] [] [] Stockpiles are stabilized with vegetation and/or mulch.
- [] [] Sediment control is installed at the toe of the slope.

#### 2. Revegetation

#### Yes No NA

- [] [] [] Temporary seedings and mulch have been applied to idle areas.
- [] [] 4 inches minimum of topsoil has been applied under permanent seedings

#### Sediment Control Practices

1. Silt Fence and Linear Barriers

#### Yes No NA

- [] [] Installed on Contour, 10 feet from toe of slope (not across conveyance channels).
- [] [] Joints constructed by wrapping the two ends together for continuous support.
- [] [] Fabric buried 6 inches minimum.
- [] [] Posts are stable, fabric is tight and without rips or frayed areas.

Sediment accumulation is ___% of design capacity.

#### CONSTRUCTION DURATION INSPECTIONS

Page 4 of _____

#### Sediment Control Practices (continued)

2. Storm Drain Inlet Protection (Use for Stone & Block; Filter Fabric; Curb; or, Excavated; Filter Sock or Manufactured practices)

#### Yes No NA

- [] [] Installed concrete blocks lengthwise so open ends face outward, not upward.
- [] [] Placed wire screen between No. 3 crushed stone and concrete blocks.
- [] [] Drainage area is 1acre or less.
- [] [] [] Excavated area is 900 cubic feet.
- [] [] Excavated side slopes should be 2:1.
- [] [] 2" x 4" frame is constructed and structurally sound.
- [] [] Posts 3-foot maximum spacing between posts.
- [] [] Fabric is embedded 1 to 1.5 feet below ground and secured to frame/posts with staples at max 8-inch spacing.
- [] [] Posts are stable, fabric is tight and without rips or frayed areas.
- [] [] Manufactured insert fabric is free of tears and punctures.
- [] [] Filter Sock is not torn or flattened and fill material is contained within the mesh sock.

Sediment accumulation ____% of design capacity.

3. Temporary Sediment Trap

#### Yes No NA

- [] [] Outlet structure is constructed per the approved plan or drawing.
- [] [] Geotextile fabric has been placed beneath rock fill.
- [] [] Sediment trap slopes and disturbed areas are stabilized.

Sediment accumulation is ___% of design capacity.

4. Temporary Sediment Basin

#### Yes No NA

- [] [] Basin and outlet structure constructed per the approved plan.
- [] [] Basin side slopes are stabilized with seed/mulch.
- [] [] Drainage structure flushed and basin surface restored upon removal of sediment basin facility.
- [] [] Sediment basin dewatering pool is dewatering at appropriate rate.

Sediment accumulation is ___% of design capacity.

<u>Note</u>: Not all erosion and sediment control practices are included in this listing. Add additional pages to this list as required by site specific design. All practices shall be maintained in accordance with their respective standards.

Construction inspection checklists for post-development stormwater management practices can be found in Appendix F of the New York Stormwater Management Design Manual.

#### CONSTRUCTION DURATION INSPECTIONS

#### **b.** Modifications to the SWPPP (To be completed as described below)

The Operator shall amend the SWPPP whenever:

- 1. There is a significant change in design, construction, operation, or maintenance which may have a significant effect on the potential for the discharge of pollutants to the waters of the United States and which has not otherwise been addressed in the SWPPP; or
- 2. The SWPPP proves to be ineffective in:
  - a. Eliminating or significantly minimizing pollutants from sources identified in the SWPPP and as required by this permit; or
  - b. Achieving the general objectives of controlling pollutants in stormwater discharges from permitted construction activity; and
- 3. Additionally, the SWPPP shall be amended to identify any new contractor or subcontractor that will implement any measure of the SWPPP.

#### **Modification & Reason:**

## **APPENDIX E**

### **Project and Owner Information**

<u>Site Data:</u> Waccabuc Country Club – Beach Club 90 Mead Street Waccabuc, New York 10597

Area: 9.1 acres ±

Owner/Applicant Information:

Waccabuc Country Club 90 Mead Street Waccabuc, New York 10597

Parties Responsible for Implementation of the Short and Long Term Maintenance Plan:

Waccabuc Country Club 90 Mead Street Waccabuc, New York 10597

and or the current owner(s) of the subject property.

Qualified Professional Responsible for Inspection of the Stormwater Pollution Prevention Plan:

Insite Engineering, Surveying & Landscape Architecture, P.C. 3 Garrett Place Carmel, New York 10512 845-225-9690

## **APPENDIX F**

NYSDEC Stormwater/Wetland Pond Construction and Maintenance Checklist

# **Stormwater/Wetland Pond Construction Inspection Checklist**

Project:
Location:
Site Status:

Date:

Time:

Inspector:

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	Comments
Pre-Construction/Materials and Equipment	-	
Pre-construction meeting		
Pipe and appurtenances on-site prior to construction and dimensions checked		
1. Material (including protective coating, if specified)		
2. Diameter		
3. Dimensions of metal riser or pre-cast concrete outlet structure		
4. Required dimensions between water control structures (orifices, weirs, etc.) are in accordance with approved plans		
5. Barrel stub for prefabricated pipe structures at proper angle for design barrel slope		
6. Number and dimensions of prefabricated anti-seep collars		
7. Watertight connectors and gaskets		
8. Outlet drain valve		
Project benchmark near pond site		
Equipment for temporary de-watering		

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	Comments
2. Subgrade Preparation	-	
Area beneath embankment stripped of all vegetation, topsoil, and organic matter		
3. Pipe Spillway Installation		
Method of installation detailed on plans		
A. Bed preparation	-	
Installation trench excavated with specified side slopes		
Stable, uniform, dry subgrade of relatively impervious material (If subgrade is wet, contractor shall have defined steps before proceeding with installation)		
Invert at proper elevation and grade		
B. Pipe placement		
Metal / plastic pipe		
1. Watertight connectors and gaskets properly installed		
2. Anti-seep collars properly spaced and having watertight connections to pipe		
3. Backfill placed and tamped by hand under "haunches" of pipe		
4. Remaining backfill placed in max. 8 inch lifts using small power tamping equipment until 2 feet cover over pipe is reached		

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	Comments
3. Pipe Spillway Installation		
Concrete pipe	1	
1. Pipe set on blocks or concrete slab for pouring of low cradle		
2. Pipe installed with rubber gasket joints with no spalling in gasket interface area		
3. Excavation for lower half of anti-seep collar(s) with reinforcing steel set		
<ol> <li>Entire area where anti-seep collar(s) will come in contact with pipe coated with mastic or other approved waterproof sealant</li> </ol>		
5. Low cradle and bottom half of anti-seep collar installed as monolithic pour and of an approved mix		
<ol><li>Upper half of anti-seep collar(s) formed with reinforcing steel set</li></ol>		
7. Concrete for collar of an approved mix and vibrated into place (protected from freezing while curing, if necessary)		
8. Forms stripped and collar inspected for honeycomb prior to backfilling. Parge if necessary.		
C. Backfilling		
Fill placed in maximum 8 inch lifts		
Backfill taken minimum 2 feet above top of anti- seep collar elevation before traversing with heavy equipment		

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	Comments
4. Riser / Outlet Structure Installation		
Riser located within embankment		
A. Metal riser		
Riser base excavated or formed on stable subgrade to design dimensions		
Set on blocks to design elevations and plumbed		
Reinforcing bars placed at right angles and projecting into sides of riser		
Concrete poured so as to fill inside of riser to invert of barrel		
B. Pre-cast concrete structure		
Dry and stable subgrade		
Riser base set to design elevation		
If more than one section, no spalling in gasket interface area; gasket or approved caulking material placed securely		
Watertight and structurally sound collar or gasket joint where structure connects to pipe spillway		
C. Poured concrete structure		
Footing excavated or formed on stable subgrade, to design dimensions with reinforcing steel set		
Structure formed to design dimensions, with reinforcing steel set as per plan		
Concrete of an approved mix and vibrated into place (protected from freezing while curing, if necessary)		
Forms stripped & inspected for "honeycomb" prior to backfilling; parge if necessary		

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	Comments
5. Embankment Construction		
Fill material		
Compaction		
Embankment		
1. Fill placed in specified lifts and compacted with appropriate equipment		
2. Constructed to design cross-section, side slopes and top width		
3. Constructed to design elevation plus allowance for settlement		
6. Impounded Area Construction		
Excavated / graded to design contours and side slopes		
Inlet pipes have adequate outfall protection		
Forebay(s)		
Pond benches		
7. Earth Emergency Spillway Construction		
Spillway located in cut or structurally stabilized with riprap, gabions, concrete, etc.		
Excavated to proper cross-section, side slopes and bottom width		
Entrance channel, crest, and exit channel constructed to design grades and elevations		

CONSTRUCTION SEQUENCE	Satisfactory / Unsatisfactory	Comments	
8. Outlet Protection			
A. End section			
Securely in place and properly backfilled			
B. Endwall			
Footing excavated or formed on stable subgrade, to design dimensions and reinforcing steel set, if specified			
Endwall formed to design dimensions with reinforcing steel set as per plan			
Concrete of an approved mix and vibrated into place (protected from freezing, if necessary)			
Forms stripped and structure inspected for "honeycomb" prior to backfilling; parge if necessary			
C. Riprap apron / channel			
Apron / channel excavated to design cross- section with proper transition to existing ground			
Filter fabric in place			
Stone sized as per plan and uniformly place at the thickness specified			
9. Vegetative Stabilization			
Approved seed mixture or sod			
Proper surface preparation and required soil amendments			
Excelsior mat or other stabilization, as per plan			

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	Comments
10. Miscellaneous		
Drain for ponds having a permanent pool		
Trash rack / anti-vortex device secured to outlet structure		
Trash protection for low flow pipes, orifices, etc.		
Fencing (when required)		
Access road		
Set aside for clean-out maintenance		
11. Stormwater Wetlands		
Adequate water balance		
Variety of depth zones present		
Approved pondscaping plan in place Reinforcement budget for additional plantings		
Plants and materials ordered 6 months prior to construction		
Construction planned to allow for adequate planting and establishment of plant community (April-June planting window)		
Wetland buffer area preserved to maximum extent possible		

# Comments:

## Actions to be Taken:

## 

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
1. Embankment and emergency spillway (Annual, After	r Major Storms)	
1. Vegetation and ground cover adequate		
2. Embankment erosion		
3. Animal burrows		
4. Unauthorized planting		
5. Cracking, bulging, or sliding of dam		
a. Upstream face		
b. Downstream face		
c. At or beyond toe		
downstream		
upstream		
d. Emergency spillway		
6.Pond, toe & chimney drains clear and functioning		
7.Seeps/leaks on downstream face		
8.Slope protection or riprap failure		
9. Vertical/horizontal alignment of top of dam "As-Built"		

# Stormwater Pond/Wetland Operation, Maintenance and Management Inspection Checklist

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
10. Emergency spillway clear of obstructions and debris		
11. Other (specify)		
2. Riser and principal spillway (Annual)		
Type: Reinforced concrete      Corrugated pipe      Masonry      1. Low flow orifice obstructed		
<ol> <li>Low flow trash rack.</li> <li>a. Debris removal necessary</li> </ol>		
b. Corrosion control		
<ol> <li>Weir trash rack maintenance</li> <li>a. Debris removal necessary</li> </ol>		
b. corrosion control		
4. Excessive sediment accumulation insider riser		
5. Concrete/masonry condition riser and barrels a. cracks or displacement		
b. Minor spalling (<1" )		
c. Major spalling (rebars exposed)		
d. Joint failures		
e. Water tightness		
6. Metal pipe condition		
7. Control valve a. Operational/exercised		
b. Chained and locked		
8. Pond drain valve a. Operational/exercised		
b. Chained and locked		
9. Outfall channels functioning		
10. Other (specify)		

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
3. Permanent Pool (Wet Ponds) (monthly	()	
1. Undesirable vegetative growth		
2. Floating or floatable debris removal required		
3. Visible pollution		
4. Shoreline problem		
5. Other (specify)		
4. Sediment Forebays		
1.Sedimentation noted		
2. Sediment cleanout when depth < 50% design depth		
5. Dry Pond Areas		
1. Vegetation adequate		
2. Undesirable vegetative growth		
3. Undesirable woody vegetation		
4. Low flow channels clear of obstructions		
5. Standing water or wet spots		
6. Sediment and / or trash accumulation		
7. Other (specify)		
6. Condition of Outfalls (Annual , After Major Storms)		
1. Riprap failures		
2. Slope erosion		
3. Storm drain pipes		
4.Endwalls / Headwalls		
5. Other (specify)		
7. Other (Monthly)		
1. Encroachment on pond, wetland or easement area		

Maintenance Item	Satisfactory/ Unsatisfactory	Comments
2. Complaints from residents		
3.Aesthetics a. Grass growing required		
b. Graffiti removal needed		
c. Other (specify)		
4. Conditions of maintenance access routes.		
5. Signs of hydrocarbon build-up		
6. Any public hazards (specify)		
8. Wetland Vegetation (Annual)		
<ol> <li>Vegetation healthy and growing Wetland maintaining 50% surface area coverage of wetland plants after the second growing season.</li> <li>(If unsatisfactory, reinforcement plantings needed)</li> </ol>		
<ul> <li>2. Dominant wetland plants:</li> <li>Survival of desired wetland plant species</li> <li>Distribution according to landscaping plan?</li> <li>3. Evidence of invasive species</li> </ul>		
4. Maintenance of adequate water depths for desired wetland plant species		
5. Harvesting of emergent plantings needed		
6. Have sediment accumulations reduced pool volume significantly or are plants "choked" with sediment		
7. Eutrophication level of the wetland.		
8. Other (specify)		

# Comments:

# Actions to be Taken:

## **APPENDIX G**

**Pipe Sizing Calculations** 



DRAINAGE SYSTEM CALCULATIONS Design Storm: 100-Year PROJECT: Waccabuc CC - Beach Club ImprovementsJOB NUMBER:20228.100BY:EJPDATE: 6-28-2022

STRU	CTURE	IMPER	VIOUS	AREA	PERV	IOUS	AREA		TIME O	F CONC	5. (min.)		Q (cf	s)		PIF	E DESI	GN	
								CA				I							
FROM	ТО	A (ac.)	С	CA	A (ac.)	С	CA		INLET	PIPE	TOTAL		DESIGN	CAP.	V(ft/s)	n	s (%)	L (ft)	DIA (in)
DI 12	DI 11	0.04	0.9	0.04	0.01	0.3	0.00	0.04	6	-	6	9.2	0.4	1.4	3.3	0.012	1.1	46	8
DI 11	DMH 10	0.16	0.9	0.14	0.05	0.3	0.02	0.22	6	-	6	9.2	2.0	4.0	5.2	0.012	1.1	82	12
DMH 10	ES 9B	0.00	0.9	0.00	0.00	0.3	0.00	0.22	6	-	6	9.2	2.0	3.9	5.0	0.012	1.0	68	12
DI 11A	DI 11	0.00	0.9	0.00	0.08	0.3	0.02	0.02	6	-	6	9.2	0.2	3.1	4.9	0.012	5.6	36	8
OS 1.1P	ES 9A								PIPE	SIZED	IN HYDF	ROCA	D						
DI 8	DI 7	0.09	0.9	0.08	0.00	0.3	0.00	0.08	6	-	6	9.2	0.7	6.6	5.6	0.012	2.9	17	12
DI 7	DI 6	0.03	0.9	0.03	0.10	0.3	0.03	0.14	6	-	6	9.2	1.3	15.2	11.8	0.012	15.4	26	12
DI 6	DMH 5	0.00	0.9	0.00	0.11	0.3	0.03	0.17	6	-	6	9.2	1.6	14.1	11.9	0.012	13.4	32	12
DMH 5	DMH 4	0.00	0.9	0.00	0.00	0.3	0.00	0.98	6	-	6	9.2	9.0	16.6	13.8	0.012	5.6	63	15
DMH 4	DMH 3	0.00	0.9	0.00	0.00	0.3	0.00	0.98	10	-	10	7.5	7.4	22.6	16.5	0.012	10.4	68	15
DMH 3	DMH 2A	0.00	0.9	0.00	0.00	0.3	0.00	0.98	10	-	10	7.5	7.4	26.7	18.6	0.012	14.6	91	15
DMH 2A	ES 2	0.00	0.9	0.00	0.00	0.3	0.00	0.98	10	-	10	7.5	7.4	13.8	11.5	0.012	3.9	46	15
SDI 5A	DMH 5	0.80	0.9	0.72	0.30	0.3	0.09	0.81	10	-	10	7.5	6.1	7.0	6.4	0.012	1.0	67	15

## **APPENDIX H**

Draft NYSDEC Notice of Intent and MS4 SWPPP Acceptance Form

## NOTICE OF INTENT



## New York State Department of Environmental Conservation

### **Division of Water**

625 Broadway, 4th Floor



Albany, New York 12233-3505

Stormwater Discharges Associated with <u>Construction Activity</u> Under State Pollutant Discharge Elimination System (SPDES) General Permit # GP-0-20-001 All sections must be completed unless otherwise noted. Failure to complete all items may result in this form being returned to you, thereby delaying your coverage under this General Permit. Applicants must read and understand the conditions of the permit and prepare a Stormwater Pollution Prevention Plan prior to submitting this NOI. Applicants are responsible for identifying and obtaining other DEC permits that may be required.

# -IMPORTANT-

## RETURN THIS FORM TO THE ADDRESS ABOVE

OWNER/OPERATOR MUST SIGN FORM

						C	)wn	er/	'Op	era	ito	r 1	nf	or	nat	tio	n												$\searrow$
Owner/Operator	(Cor	npan	y N	am	e/P	riv	ate	e O	wne	er	Nar	ne/	Mu	nic	rip	al	it	y 1	Jan	le)	 								
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Owner/Operator	Cont	tact	Pe	rs	on i	Las	t 1	Jam	e	(NO	T (	CON	SU	LTA	NT	])		[			1	1		1	1	1			
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Owner/Operator	Cont	tact	Pe	rs	on	Fir	st	Na	me		1	1		1 1							1	1		1	1	1			
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Project Site Informat	tion
Project/Site Name         W       a       c       c       a       b       u       c       C       C       B       e       a       c       h       C       l       u       b	I m p r o v e m e n t s
Street Address (NOT P.O. BOX)         2       0       P       e       r       c       h       B       a       y       R       o       a       d       Image: state st	
Side of Street © North O South O East O West	
City/Town/Village (THAT ISSUES BUILDING PERMIT)          T o w n o f       L e w i s b o r o	
State         Zip         County           N Y         1 0 5 9 7 -         Westchest	DEC Region
Name of Nearest Cross Street PostOfficeRoad	
Distance to Nearest Cross Street (Feet)	Project In Relation to Cross Street <b>North</b> O South O East O West
Tax Map Numbers Section-Block-Parcel 32.4-3-14	Tax Map Numbers

1. Provide the Geographic Coordinates for the project site in NYTM Units. To do this you **must** go to the NYSDEC Stormwater Interactive Map on the DEC website at:

#### www.dec.ny.gov/imsmaps/stormwater/viewer.htm

Zoom into your Project Location such that you can accurately click on the centroid of your site. Once you have located your project site, go to the tool boxes on the top and choose "i"(identify). Then click on the center of your site and a new window containing the X, Y coordinates in UTM will pop up. Transcribe these coordinates into the boxes below. For problems with the interactive map use the help function.

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ΥC	loor	dina	ates	(N	orth	ning	)
4	5	7	2	5	6	9	

2. What is the nature of this construction project?

O New Construction
O Redevelopment with increase in impervious area
O Redevelopment with no increase in impervious area

3. Select the predominant land use for bot SELECT ONLY ONE CHOICE FOR EACH	th pre and post development conditions.
Pre-Development Existing Land Use	Post-Development Future Land Use
⊖ FOREST	O SINGLE FAMILY HOME Number_of Lots
○ PASTURE/OPEN LAND	○ SINGLE FAMILY SUBDIVISION
○ CULTIVATED LAND	○ TOWN HOME RESIDENTIAL
○ SINGLE FAMILY HOME	○ MULTIFAMILY RESIDENTIAL
$\bigcirc$ SINGLE FAMILY SUBDIVISION	○ INSTITUTIONAL/SCHOOL
$\bigcirc$ TOWN HOME RESIDENTIAL	○ INDUSTRIAL
$\bigcirc$ MULTIFAMILY RESIDENTIAL	○ COMMERCIAL
○ INSTITUTIONAL/SCHOOL	O MUNICIPAL
○ INDUSTRIAL	○ ROAD/HIGHWAY
○ COMMERCIAL	○ RECREATIONAL/SPORTS FIELD
○ ROAD/HIGHWAY	○ BIKE PATH/TRAIL
○ RECREATIONAL/SPORTS FIELD	○ LINEAR UTILITY (water, sewer, gas, etc.)
○ BIKE PATH/TRAIL	O PARKING LOT
○ LINEAR UTILITY	○ CLEARING/GRADING ONLY
○ PARKING LOT	$\bigcirc$ DEMOLITION, NO REDEVELOPMENT
⊘ OTHER	$\bigcirc$ WELL DRILLING ACTIVITY *(Oil, Gas, etc.)
Beach Club	OTHER BeachClub

*Note: for gas well drilling, non-high volume hydraulic fractured wells only

4.	. In accordance with the larger common plan of development or sale, enter the total project site area; the total area to be disturbed; existing impervious area to be disturbed (for redevelopment activities); and the future impervious area constructed within the disturbed area. (Round to the nearest tenth of an acre.)	
		Future Impervious Area Within Disturbed Area
5.	. Do you plan to disturb more than 5 acres of soil at any one time?	Yes 🖌 No
6.	Indicate the percentage of each Hydrologic Soil Group(HSG) at the         A       B       C       D         0 %       5 0 %       5 0 %       0 %	
7.	. Is this a phased project?	○Yes ⊘No
8.	. Enter the planned start and end dates of the disturbance activities.	Date / 3 0 / 2 0 2 3

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	existing impervious cover and where the Soil Slope Phase is	🔾 Yes	🖉 No
	identified as an E or F on the USDA Soil Survey?		
	If Yes, what is the acreage to be disturbed?		

14. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent O Yes O No area?

15.	Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, O Yes @ culverts, etc)?	No (	) Unknown
16.	What is the name of the municipality/entity that owns the separate system?	storm	sewer
17.	Does any runoff from the site enter a sewer classified O Yes @ as a Combined Sewer?	No (	Unknown
18.	Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law?	0 Y	es 🕐 No
19.	Is this property owned by a state authority, state agency, federal government or local government?	0 <b>y</b>	es 🕐 No
20.	Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup Agreement, etc.)	0 Y	es 🕐 No
21.	Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book)?	Ø У	es O No
22.	Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and Quantity Control practices/techniques)? If No, skip questions 23 and 27-39.	• Y	es O No
23.	Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS Stormwater Management Design Manual?	• Y	es O No

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### SWPPP Preparer Certification

I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) for this project has been prepared in accordance with the terms and conditions of the GP-0-20-001. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of this permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

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25.	Has a construction sequence schedule for the planned management practices been prepared?	Ø Yes	() No
26.	Select <b>all</b> of the erosion and sediment control practices that will employed on the project site:	be	

### Temporary Structural

Check Dams

Construction Road Stabilization

✔ Dust Control

Earth Dike

✔ Level Spreader

Perimeter Dike/Swale

Pipe Slope Drain

Portable Sediment Tank

Rock Dam

- Sediment Basin
- ✓ Sediment Traps
- ✓ Silt Fence
- Stabilized Construction Entrance
- Storm Drain Inlet Protection
- Straw/Hay Bale Dike

Temporary Access Waterway Crossing

Temporary Stormdrain Diversion

Temporary Swale

Turbidity Curtain

Water bars

### Biotechnical

Brush Matting

Wattling

Other

#### Vegetative Measures

Brush Matting Dune Stabilization Grassed Waterway Mulching

Protecting Vegetation

Recreation Area Improvement

- ✔ Seeding
  - Sodding

Straw/Hay Bale Dike

Streambank Protection

- Temporary Swale
- Topsoiling
   Vegetating Waterways

#### Permanent Structural

Debris Basin

Diversion

Grade Stabilization Structure

Land Grading

Lined Waterway (Rock)

Paved Channel (Concrete)

Paved Flume

✓ Retaining Wall

Riprap Slope Protection

Rock Outlet Protection
 Streambank Protection

		_																		
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#### Post-construction Stormwater Management Practice (SMP) Requirements

<u>Important</u>: Completion of Questions 27-39 is not required if response to Question 22 is No.

27.	Identify all site planning practices that were used to prepare the final site plan/layout for the project.
	Preservation of Undisturbed Areas
	Preservation of Buffers
	✓ Reduction of Clearing and Grading
	Locating Development in Less Sensitive Areas
	Roadway Reduction
	Sidewalk Reduction
	<pre>     Driveway Reduction </pre>
	Cul-de-sac Reduction
	Building Footprint Reduction
	Parking Reduction

- 27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).

  - O Compacted areas were considered as impervious cover when calculating the WQv Required, and the compacted areas were assigned a post-construction Hydrologic Soil Group (HSG) designation that is one level less permeable than existing conditions for the hydrology analysis.
- 28. Provide the total Water Quality Volume (WQv) required for this project (based on final site plan/layout).

Tot				-		
	0	-	0	4	6	acre-feet

29. Identify the RR techniques (Area Reduction), RR techniques(Volume Reduction) and Standard SMPs with RRv Capacity in Table 1 (See Page 9) that were used to reduce the Total WQv Required(#28).

Also, provide in Table 1 the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

**Note:** Redevelopment projects shall use Tables 1 and 2 to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

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Table 1 -	Run	off Rec	duction	(RR) 1	lechniques
	and	Standa	ard Stor	rmwater	Management
	Pra	ctices	(SMPs)		

		Contributing		Cotal Co				
RR Techniques (Area Reduction)		a (acres)	<u></u>	pervious		Tea	a (a	STES/
Conservation of Natural Areas (RR-1)			and/or					
Sheetflow to Riparian Buffers/Filters Strips (RR-2)		-	and/or					
✔ Tree Planting/Tree Pit (RR-3)			and/or	0	-	0	3	0
Disconnection of Rooftop Runoff (RR-4)		•	and/or		-			
RR Techniques (Volume Reduction)								
Vegetated Swale (RR-5)					•			
Rain Garden (RR-6)	•••••				-			
Stormwater Planter (RR-7)		••••••••••						
🖌 Rain Barrel/Cistern (RR-8)		••••••••••		0		0	9	0
Porous Pavement (RR-9)	•••••							
Green Roof (RR-10)								
Standard SMPs with RRv Capacity								
Infiltration Trench (I-1) ·····		•••••	•••••		-			
Infiltration Basin (I-2) ·····					-			
Dry Well (I-3)	• • • • • • •				-			
Underground Infiltration System (I-4)		•••••			-			
Bioretention (F-5)					-			
Dry Swale (O-1)					•			
Standard SMPs								
✓ Micropool Extended Detention (P-1)			••••	0	-	2	0	4
Wet Pond (P-2)	• • • • • • •				-			
Wet Extended Detention (P-3) ·····	• • • • • • •		••••		-			
Multiple Pond System (P-4)					-			
Pocket Pond (P-5) ·····	•••••	•••••	•••••					
Surface Sand Filter (F-1)		••••••						
Underground Sand Filter (F-2)								
				1 1 1				1

 Perimeter Sand Filter (F-3)
 •

 Organic Filter (F-4)
 •

 Shallow Wetland (W-1)
 •

 Extended Detention Wetland (W-2)
 •

 Pond/Wetland System (W-3)
 •

 Pocket Wetland (W-4)
 •

 Wet Swale (0-2)
 •

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Table 2 -       Alternative SMPs         (DO NOT INCLUDE PRACT         USED FOR PRETREATMENT	T ONLY)
Alternative SMP	Total Contributing Impervious Area(acres)
<pre>O Hydrodynamic</pre>	·····
Provide the name and manufacturer of the Alternative S proprietary practice(s)) being used for WQv treatment. Name Manufacturer Manufacturer Manufacturer Manufacturer Manufacturer Manufacturer Manufacturer Mote: Redevelopment projects which do not use RR technicuse questions 28, 29, 33 and 33a to provide SMPs WQv required and total WQv provided for the projects	iques, shall used, total
30. Indicate the Total RRv provided by the RR techni Standard SMPs with RRv capacity identified in qu Total RRv provided          0       0       2       3	
31. Is the Total RRv provided (#30) greater than or total WQv required (#28). If Yes, go to question 36. If No, go to question 32.	equal to the O <b>Yes @ No</b>
32. Provide the Minimum RRv required based on HSG. [Minimum RRv Required = (P) (0.95) (Ai)/12, Ai=(S)	(Aic)]
Minimum RRv Required	
<ul> <li>32a. Is the Total RRv provided (#30) greater than or Minimum RRv Required (#32)?</li> <li>If Yes, go to question 33.</li> <li>Note: Use the space provided in question #39 specific site limitations and justification f 100% of WQv required (#28). A detailed evalu specific site limitations and justification f 100% of the WQv required (#28) must also be i SWPPP.</li> <li>If No, sizing criteria has not been met, so NOI processed. SWPPP preparer must modify design to processed.</li> </ul>	

33. Identify the Standard SMPs in Table 1 and, if applicable, the Alternative SMPs in Table 2 that were used to treat the remaining total WQv(=Total WQv Required in 28 - Total RRv Provided in 30).

Also, provide in Table 1 and 2 the total <u>impervious</u> area that contributes runoff to each practice selected.

Note: Use Tables 1 and 2 to identify the SMPs used on Redevelopment projects.

33a. Indicate the Total WQv provided (i.e. WQv treated) by the SMPs identified in question #33 and Standard SMPs with RRv Capacity identified in question 29. WQv Provided ³ acre-feet 0 0 2 Note: For the standard SMPs with RRv capacity, the WQv provided by each practice = the WQv calculated using the contributing drainage area to the practice - RRv provided by the practice. (See Table 3.5 in Design Manual) 34. Provide the sum of the Total RRv provided (#30) and 0 0 4 6 the WQv provided (#33a). 35. Is the sum of the RRv provided (#30) and the WQv provided (#33a) greater than or equal to the total WQv required (#28)? 🕑 Yes 🛛 🔿 No If Yes, go to question 36. If No, sizing criteria has not been met, so NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria. Provide the total Channel Protection Storage Volume (CPv) required and 36. provided or select waiver (36a), if applicable. CPv Required **CPv** Provided 0 5 5  $\cap$ 8 0  $\cap$ 8 acre-feet acre-feet 36a. The need to provide channel protection has been waived because: O Site discharges directly to tidal waters or a fifth order or larger stream. O Reduction of the total CPv is achieved on site through runoff reduction techniques or infiltration systems.

37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) control criteria or select waiver (37a), if applicable.

#### Total Overbank Flood Control Criteria (Qp)

Pre-Development	Post-development
Total Extreme Flood Control	Criteria (Qf)
Pre-Development	Post-development
3 2 . 9 0 CFS	32.39 _{CFS}

37a.	The need to meet the Qp and Qf criteria has been waived because:
	O Site discharges directly to tidal waters
	or a fifth order or larger stream.
	$\bigcirc$ Downstream analysis reveals that the Qp and Qf
	controls are not required

38. Has a long term Operation and Maintenance Plan for the post-construction stormwater management practice(s) been @ developed?

🕑 Yes 🛛 🔿 No

If Yes, Identify the entity responsible for the long term  $\ensuremath{\mathsf{Operation}}$  and Maintenance

W	а	1	С	С	a	b	u	С	С	0	u	n	t	У	С	1	u	b								

### 39. Use this space to summarize the specific site limitations and justification for not reducing 100% of WQv required(#28). (See question 32a) This space can also be used for other pertinent project information.

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40. Identify other DEC permits, existing and new, that are required for this project/facility.

Air Pollution Control Coastal Erosion Hazardous Waste Long Island Wells Mined Land Reclamation Solid Waste Navigable Waters Protection / Article 15 Water Quality Certificate Dam Safety Water Supply Freshwater Wetlands/Article 24 Tidal Wetlands Wild, Scenic and Recreational Rivers Stream Bed or Bank Protection / Article 15 Endangered or Threatened Species (Incidental Take Permit) Individual SPDES SPDES Multi-Sector GP NYR Other ✓ None

41.	Does this project require a US Army Corps of Engineers Wetland Permit? If Yes, Indicate Size of Impact.	O Yes	🕐 No
42.	Is this project subject to the requirements of a regulated, traditional land use control MS4? (If No, skip question 43)	Ø Yes	O No
43.	Has the "MS4 SWPPP Acceptance" form been signed by the principal executive officer or ranking elected official and submitted along with this NOI?	Ø Yes	O No
44.	If this NOI is being submitted for the purpose of continuing or trans coverage under a general permit for stormwater runoff from constructi activities, please indicate the former SPDES number assigned. N Y R	2	

#### Owner/Operator Certification

I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I hereby certify that this document and the corresponding documents were prepared under my direction or supervision. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting this NOI and can be as long as sixty (60) business days as provided for in the general permit. I also understand that, by submitting this NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction, and agreeing to comply with all the terms and conditions of the general permit for which this NOI is being submitted.

MI
Date

NYS Department of Environmental ConservationDivision of Water 625 Broadway, 4th Floor Albany, New York 12233-3505				
MS4 Stormwater Pollution Prevention Plan (SWPPP) Acceptance				
Form				
for Construction Activities Seeking Authorization Under SPDES General Permit *(NOTE: Attach Completed Form to Notice Of Intent and Submit to Address Above)				
I. Project Owner/Operator Information				
1. Owner/Operator Name: Waccabuc Country Club				
2. Contact Person: John Assumma				
3. Street Address: 90 Mead Street				
4. City/State/Zip: Waccabuc, New York 10597				
II. Project Site Information				
5. Project/Site Name: Waccabuc Country Club - Beach Club Improvements				
6. Street Address: 20 Perch Bay Road				
7. City/State/Zip: Waccabuc, New York 10597				
III. Stormwater Pollution Prevention Plan (SWPPP) Review and Acceptance Information				
8. SWPPP Reviewed by:				
9. Title/Position:				
10. Date Final SWPPP Reviewed and Accepted:				
IV. Regulated MS4 Information				
11. Name of MS4: Town of Lewisboro				
12. MS4 SPDES Permit Identification Number: NYR20A				
13. Contact Person:				
14. Street Address:				
15. City/State/Zip:				
16. Telephone Number:				

# MS4 SWPPP Acceptance Form - continued

# V. Certification Statement - MS4 Official (principal executive officer or ranking elected official) or Duly Authorized Representative

I hereby certify that the final Stormwater Pollution Prevention Plan (SWPPP) for the construction project identified in question 5 has been reviewed and meets the substantive requirements in the SPDES General Permit For Stormwater Discharges from Municipal Separate Storm Sewer Systems (MS4s). Note: The MS4, through the acceptance of the SWPPP, assumes no responsibility for the accuracy and adequacy of the design included in the SWPPP. In addition, review and acceptance of the SWPPP by the MS4 does not relieve the owner/operator or their SWPPP preparer of responsibility or liability for errors or omissions in the plan.

Printed Name:

Title/Position:

Signature:

Date:

VI. Additional Information

(NYS DEC - MS4 SWPPP Acceptance Form - January 2015)

# **APPENDIX I**

## **Temporary Sediment Trap Sizing Calculations**

3,600 CF minimum of storage required for each acre of contributing area

Sediment Trap #	Contributing Area	Minimum Volume Required	Volume Provided
	(Acres)	(cf)	(cf)
А	0.5	1,800	2,016

## **APPENDIX J**

**Draft Stormwater Maintenance Agreement** 

### DECLARATION OF COVENANTS, CONDITIONS, AND RESTRICTIONS FOR DREW REALTY LLC

**DECLARATION** made as of the <u>day of</u>, 2023, by Waccabuc Country Club with an address at 90 Mead Street, Waccabuc, NY 10597 (hereinafter referred to as the "Declarant").

#### WITNESSETH:

WHEREAS, Declarant is the owner of all that certain lot, piece or parcel of land situate, lying and being in the Town of Lewisboro, County of Westchester and State of New York, being designated as Section 32.4, Block 3 and Lot 13 & 14 as described in a deed recorded in the Westchester County Clerk's office as document control number

_____ attached hereto as Exhibit 1 (the "Property"); and

WHEREAS, the Watershed Regulations require Declarant to prepare a Stormwater Pollution Prevention Plan ("SWPPP") and submit the SWPPP to the New York City Department of Environmental Protection ("DEP") for its review and approval so that stormwater generated by precipitation during and after soil disturbing activities and runoff from newly created impervious surfaces is captured and treated, thus reducing or eliminating a pollution discharge; and

WHEREAS, Declarant has submitted a SWPPP application to DEP for the Property described above, <u>Waccabuc Country Club – Beach Club</u>, and received an approval from DEP for such SWPPP, dated ______, such SWPPP approval and the maintenance obligations being attached hereto as Exhibit 2; and

WHEREAS, Declarant desires to declare the following covenants, conditions and restrictions to govern the future development, use and maintenance of any lots that are part of the Property that may be conveyed to future owners, including the Declarant's respective heirs, successors, and assigns, and to subject any deed of conveyance of any such lots to this Declaration, by reference thereto, to the covenants, conditions and restrictions described herein,

**NOW, THEREFORE**, Declarant hereby declares that the Property shall be held, sold, conveyed, transferred and occupied subject to the following covenants, conditions, and restrictions which are for the benefit of the City of New York as well as for the owners of the Property and which shall be perpetual so long as the provisions of the SWPPP continue to be required by the Watershed Regulations, shall run with the Property and be binding on the Declarant, its heirs, successors and assigns and be binding upon each successive owner of any Property parcel or lot described in the subdivision plan and the heirs, successors and assigns of each subsequent party having or acquiring any right, title or interest in the Property or any part thereof.

1. Declarant hereby acknowledges, covenants, warrants, and represents that it shall install and maintain any and all erosion and sediment controls and stormwater management practices on the Property in accordance with the SWPPP approved by DEP, dated _____, and any and all amendments to the SWPPP that may be required and that DEP may approve.

- 2. Declarant's installation and maintenance of the erosion and sediment controls and stormwater management practices shall be for the benefit of the City of New York as well as for the owners of the Property.
- 3. Declarant's obligation to install and maintain any and all erosion and sediment controls and stormwater management practices on the Property in accordance with the DEP-approved SWPPP and any and all amendments to the SWPPP that DEP may approve shall be perpetual so long as the provisions of the SWPPP continue to be required by the Watershed Regulations.
- 4. Declarant hereby acknowledges, covenants and warrants that this Property shall be subject to the maintenance obligations set forth and described in the SWPPP, with respect to any stormwater management practices or treatment of runoff located on areas commonly owned by multiple property owners or a homeowners' association in the subdivision.
- 5. Declarant hereby covenants, warrants, and represents that any lease, mortgage, subdivision, or other transfer of the Property, or any interest therein, shall be subject to the restrictive covenants contained herein pertaining to the installation and maintenance of erosion and sediment control and stormwater management practices, and any deed, mortgage, or other instrument of conveyance shall be subject to and, specifically refer to, the attached SWPPP approval and shall specifically state that the interest thereby conveyed is subject to the covenants and restrictions contained herein and therein.
- 6. These covenants, conditions and restrictions shall be recorded at the Office of the County Clerk, shall run with the land and shall apply to, inure to the benefit of, and bind the Declarant and all subsequent heirs, executors, administrators, successors and assigns.

**IN WITNESS WHEREOF**, Declarant has executed this document on the date first above written.

Signature

STATE OF NEW YORK	)
	)
COUNTY OF	)

On ______, 20___, before me, the undersigned, a Notary Public in and for said State, personally appeared ______, personally known to me or proved to me on the basis of satisfactory evidence to be the individual whose name is subscribed to the within instrument and acknowledged to me that he executed the same in his/her capacity, and that by his/her signature on the instrument, the individual, or the person upon behalf of which the individual acted, executed the instrument.

Notary Public

Exhibit 1

Exhibit 2

	PERMANENT STORMWATER FACILITIES MAINTENANCE SCHEDULE					
PRACTICE/FACILITY	MONTHLY	AFTER MAJOR STORM EVENTS	BI-ANNUALLY	YEARLY	EVERY 5 to 10 YEARS	
STORMWATER POND	Inspect first few months after construction for eroding soils & slumpage & repair immediately	Inspect orifices, inlets & outlets for clogging, eroding soils on the basin berm & embankments, & sources of erosion; & stabilize and/or repair immediately.	Mow berm and exterior embankments Remove debris & litter from basin & outlet structures	Inspect and maintain all safety elements including maintenance access, safety/aquatic bench,	Inspect for & remove accumulated sediment or when 50% full	
CISTERN	Inspect tanks and irrigation pump and provide maintenance as required	_	_	Inspect for & remove accumulated sediment	_	

<u>Note:</u> The party responsible for implementation of the maintenance schedule during and after construction is:

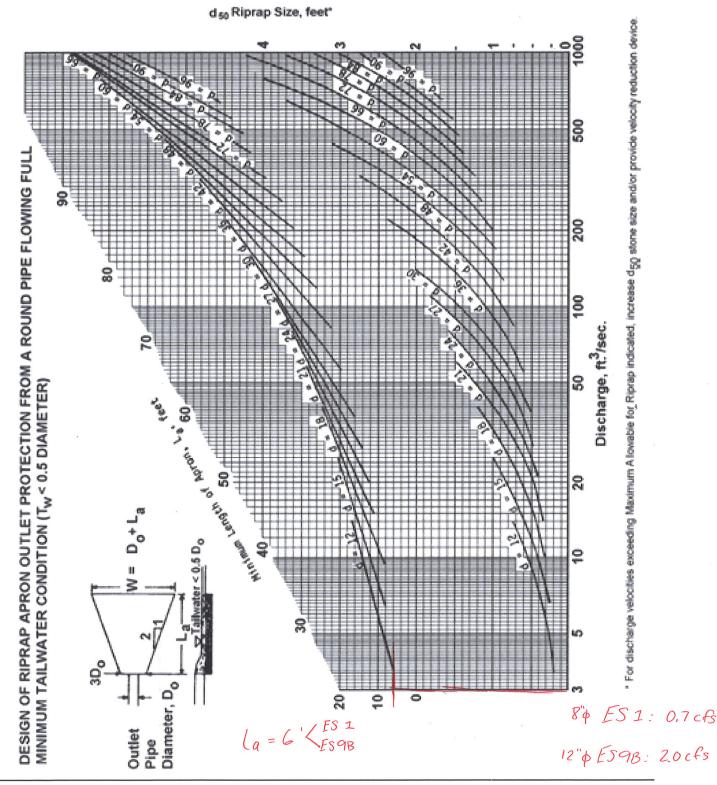
WACCABUC COUNTRY CLUB 90 MEAD STREET WACCABUC, NY 10597

#### APPENDIX K Rock Outlet Protection Sizing

Waccabuc Beach Club Improvements - Rock Outlet Protection Sizing

Figure 3.16

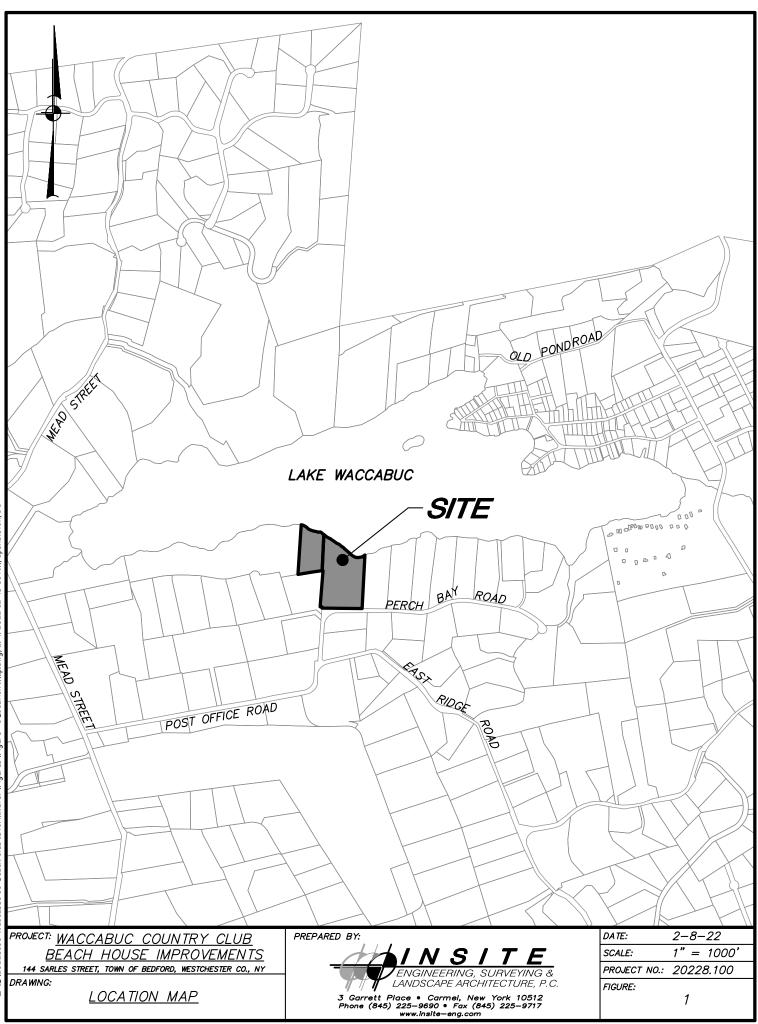
Outlet Protection Design—Minimum Tailwater Condition Chart (Design of Outlet Protection from a Round Pipe Flowing Full, Minimum Tailwater Condition: T_w < 0.5D_o) (USDA - NRCS)

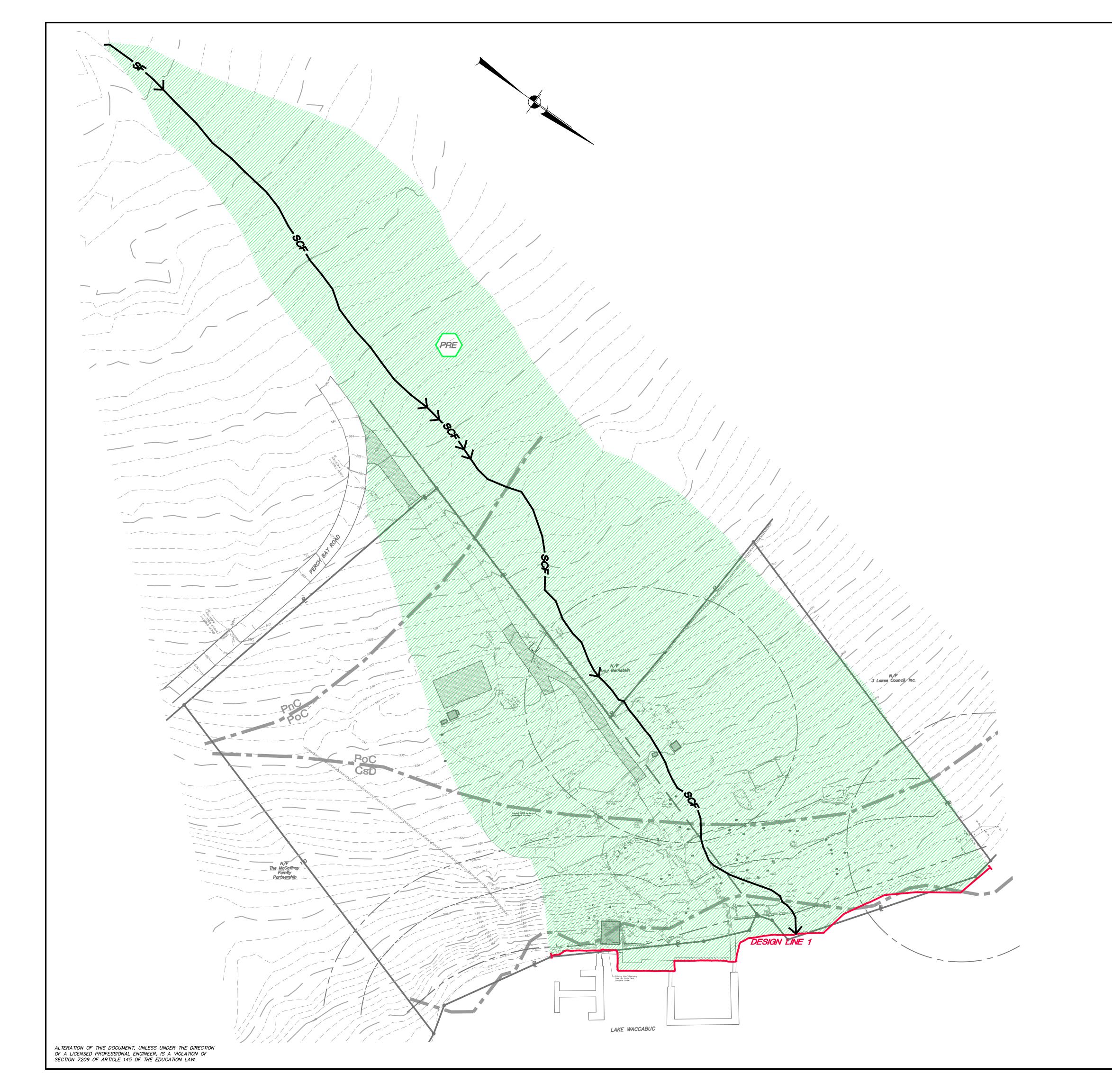


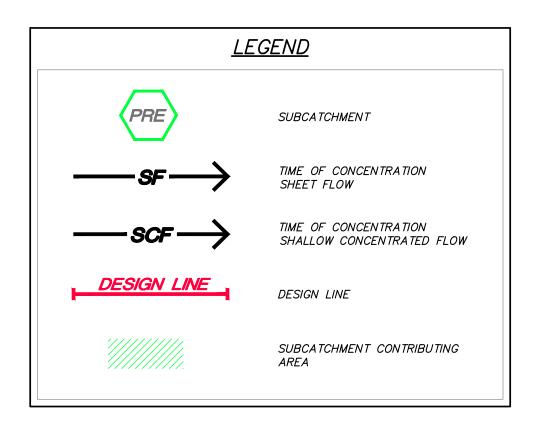
New York State Standards and Specifications For Erosion and Sediment Control Page 3.42

November 2016

# FIGURES

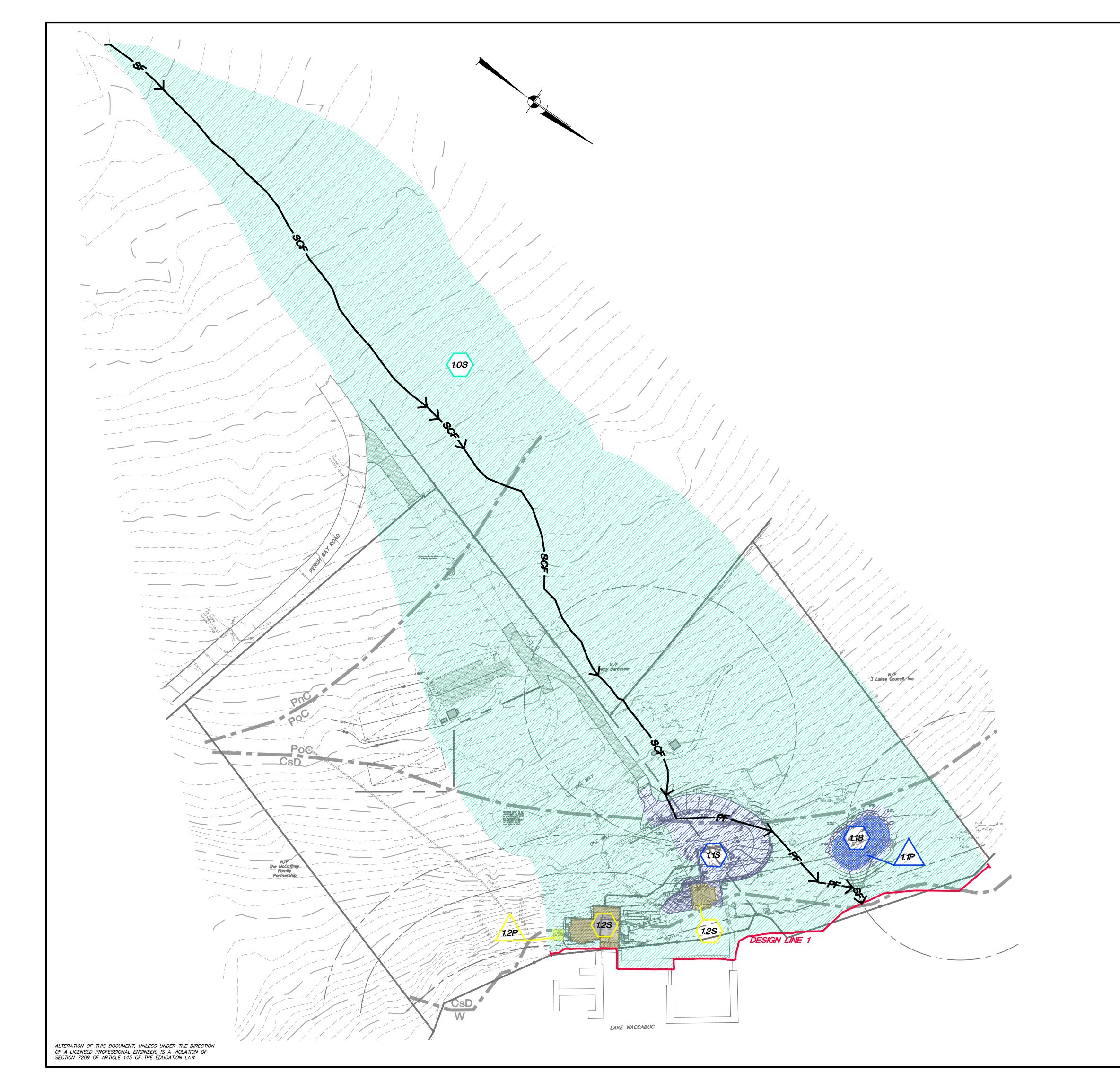


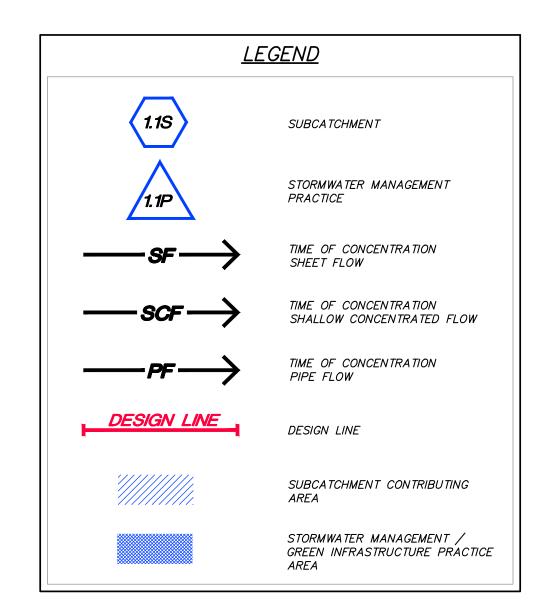




<u>SOILS LEGEND</u>						
SOILS	SOILS DESCRIPTION					
CsD	CsD Chatfield-Charlton complex, hilly, very rocky					
PnC	Paxton fine sandy loam, 8% to 15% slopes	С				
PoC	Paxton fine sandy loam, 8% to 15% slopes, very stony	С				
NRCS Soil Boundary Line						

	NO.	DATE		REVISION		BY
			ERING, SUR	VEYING &	3 Garrett Place Carmel, NY 10512 (845) 225–9690 (845) 225–9717 fr www.insite–eng.com	
	PROJE	CT:				
		<u>VACCABUC C</u> EACH CLUB				
	90 MEAD 3	ST., WACCABUC, TOWN O	F LEWISBORO, WEST	CHESTER CTY, NY		
	DRAWI	NG:				
		<u>PRE–DEV</u>	<u>ELOPMENT</u>	-		
GRAPHIC SCALE		<u>DRAINA</u>	<u>GE MAP</u>			
30 60 120	PROJEC NUMBER		PROJECT MANAGER	Z.M.P.	DRAWING NO.	
( IN FEET )	DATE	2-8-22	DRAWN BY	J.L. T.	FIG-2	>
1 inch = $60$ ft.	SCALE	1" = 60'	CHECKED BY	<i>E.J.P</i> .	, , , , , , , , , , , , , , , , , , , ,	-





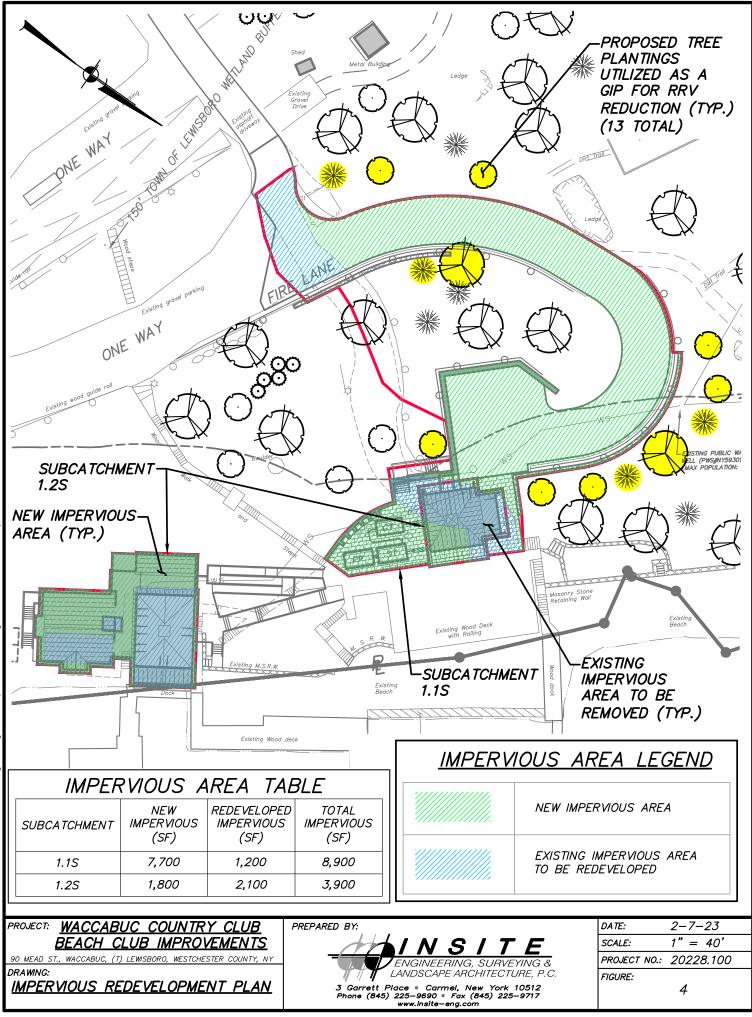
	<u>SOILS LEGEND</u>						
SOILS	DESCRIPTION	HYDROLOGICAL GROUP					
CsD	Chatfield–Charlton complex, hilly, very rocky	В					
PnC	Paxton fine sandy loam, 8% to 15% slopes	С					
PoC	Paxton fine sandy loam, 8% to 15% slopes, very stony	С					
	NRCS Soil Boundary Line						

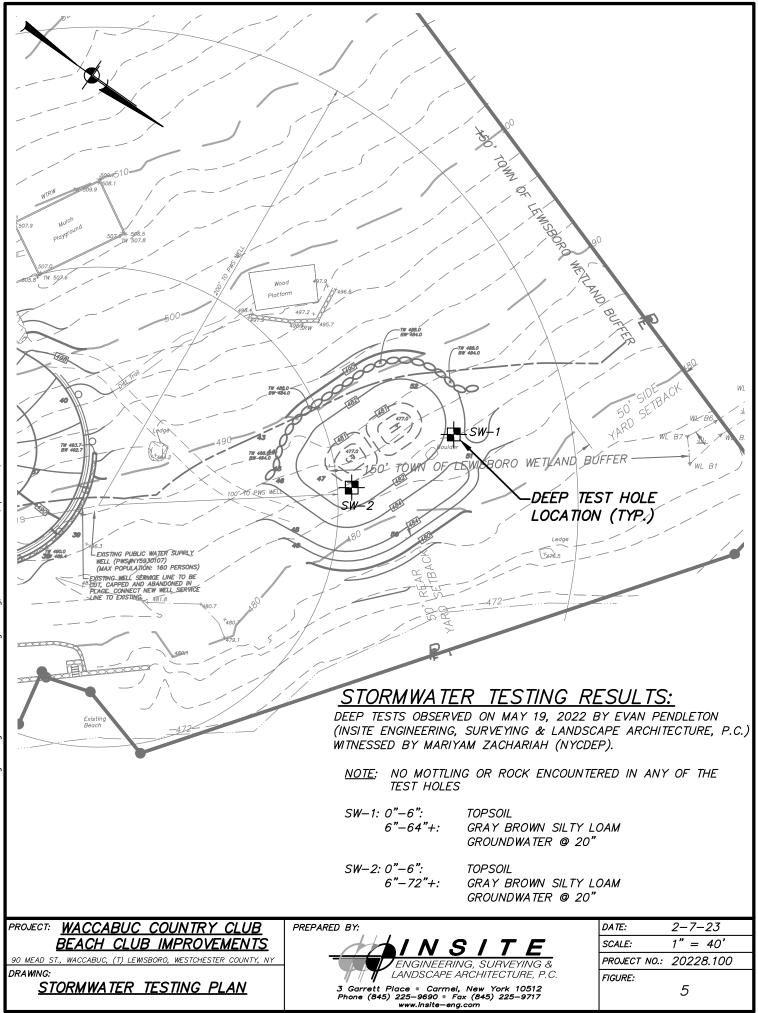
AREA TABLE					
SUBCATCHMENT	TOTAL AREA				
1.05	10.5 AC.				
1.15	20,500 S.F.				
1.25	3,900 S.F.				
1.25	3,900 S.F.				

GRAPHIC SCALE

( IN FEET ) 1 inch = 60 ft.

5	2-7-23		REVISED	PER NYCDEP COM	MENTS	EJP
4 12–9–22 REVISED PER NYCDEP COMMENTS					EJP	
3	6–28–22		REVISED	PER TOWN COMM	IENTS	EJP
2	5-31-22		REVISED	PER TOWN COMM	IENTS	EJP
1	3–29–22		REVISED	PER TOWN COMM	IENTS	EJP
NO.	DATE			REVISION		BY
-	VECT: WACCA BEACH D ST., WACCAE WING: POS	ENGINEE ANDSCAF <u>BUC CO</u> <u>CLUB I</u> BUC, TOWN OF	ERING, SUI PE ARCHITE OUNTRY MPROVEN	<u>MENTS</u> stchester cty, ny	3 Garrett Place Carmel, NY 10512 (845) 225–9690 (845) 225–9717 f www.insite–eng.com	
PROJE NUMBI		28.100	PROJECT MANAGER	Z.M.P.	DRAWING NO.	
DATE	2-8	8–22	DRAWN BY	J.L. T.	FIG-J	3
SCALE	1"	= 60'	CHECKED BY	<i>E.J.P</i> .		-





Z:\E\20228100 Waccabuc CC-Beach Club\Stormwater\Figure 5- Stormwater Testing Plan,dwg, 2/3/2023 6:50:04 AM, ependleton, 1:1

#### **Ciorsdan Conran**

Subject: Attachments: Mandia stamped PB Res 061620.pdf CO011-17-22 012351.pdf; Mandia bond escrow 041221.pdf; Mandia stamped PB Res 061620.pdf

From: Bruce Mandia <bcjjm75@gmail.com> Sent: Thursday, November 17, 2022 3:57 PM To: Ciorsdan Conran <Planning@lewisborogov.onmicrosoft.com> Subject: Re: Mandia stamped PB Res 061620.pdf

Dear Ciorsdan,

Please release 90% of the funds deposited with the town regarding the renovation of 65 Old Bedford Rd. Thank you, Bruce Mandia Sent from my iPhone

Condition #6

In accordance with Section 220-46G of the Zoning Code, the applicant shall supply a performance bond, or other form of security, to guarantee completion of project infrastructure in a sum approved by the Town Engineer based on a cost estimate to be prepared by the applicant's design engineer. The form of the bond or performance security shall be acceptable to the Planning Board Attorney. The delivery and acceptance of this security does not relieve the applicant of the obligation to complete the project infrastructure. Said bond or performance security shall provide for project infrastructure to be completed within twenty-four (24) months of commencement of work and for the retention by the Town of 10% of the originally fixed amount for a period of one(1) year after the Certificate of Occupancy has been issued.

#### TOWN OF LEWISBORO Westchester County, New York



Tel: (914) 763-5592 Fax: (914) 875-9148 Email: planning@lewisborogov.com

December 22, 2022

# VIA EMAIL AND HAND DELIVERY

**Planning Board** 

79 Bouton Road

South Salem, New York 10590

Honorable Tony Gonçalves, Supervisor and Members of the Town BoardTown of Lewisboro11 Main StreetSouth Salem, New York 10590

Re: Mandia Residences 65 Old Bedford Road Cal. #02-20 PB Authorization to Approve Release of Partial Performance Security

Dear Supervisor Gonçalves and Members of the Town Board:

I submit this letter, on behalf of the Planning Board, in connection with a request by 65 Old Bedford Road, LLC for the partial release of a performance security held by the Town guaranteeing completion of infrastructure associated with the abovereferenced project. Specifically, I request that the Town Board authorize the Planning Board to conduct a public hearing and act upon this request.

The performance security is in the form of a cash deposit held by the Town in the sum of \$46,351.00. This security was submitted to the Town in accordance with a condition of the Planning Board's Site Development Plan Approval for this project (the conversion of the former Goldens Bridge Community House into a multi-family dwelling). This approval is set forth in a Resolution adopted by the Planning Board on June 19, 2020, which states, in part:

. . . the applicant shall supply a performance bond, or other form of security, to guarantee completion of project infrastructure in a sum approved by the Town Engineer based on a cost estimate to be prepared by the applicant's design engineer. The form of the bond or performance security shall be acceptable to the Planning Board Attorney. The delivery and acceptance of this security does not relieve the applicant Honorable Tony Goncalves, Supervisor and Members of the Town Board December 22, 2022 Page 2

> of the obligation to complete the project infrastructure. Said bond or performance security shall provide for project infrastructure to be completed within twenty-four (24) months of commencement of work and for the retention by the Town of 10% of the originally fixed amount for a period of one (1) year after the Certificate of Occupancy has been issued.

A Certificate of Occupancy for the multi-family residence has been issued. Consequently, the return of 90% of this deposit (equaling \$41,715.90) is sought.

The Planning Board seeks authorization from the Town Board to hear and decide this request because of the nature and purpose of this security.

Respectfully submitted,

Janet andersen CC

Janet Andersen, Chair

cc: Gregory Folchetti, Esq. **Planning Board Members** Ms. Ciorsdan Conran Jan Johannessen, AICP, Kellard Sessions Judson K. Siebert, Esq.

## RESOLUTION ADOPTED BY THE TOWN BOARD OF THE TOWN OF LEWISBORO AT A MEETING HELD ON JANUARY 9, 2023

RESOLVED, that the Town Board of the Town of Lewisboro does authorize the Planning Board to consider and act upon 65 Old Bedford Road, LLC's request for partial release of performance bond for guarantee of completion of project infrastructure.

### STATE OF NEW YORK COUNTY OF WESTCHESTER

I, JANET L. DONOHUE, Town Clerk of the Town of Lewisboro, County of Westchester, State of New York, do hereby certify that I have compared the preceding copy of a Resolution adopted by the Town Board of the Town of Lewisboro at a meeting held on the 9th day of January, 2023, to the original thereof, and that the same is a true and exact copy of said original and of the whole thereof.

Town Clerk

Dated at South Salem, New York this 10th day of January, 2023

# TOWN OF LEWISBORO NOTICE OF PUBLIC HEARING

**NOTICE IS HEREBY GIVEN** that the Planning Board of the Town of Lewisboro, Westchester County, New York will convene a Public Hearing on Tuesday, February 28, 2023 at 7:30 p.m., or soon thereafter at the Town Offices at 79 Bouton Road, South Salem, New York, regarding the following:

#### Cal #02-20PB

Request for partial release of a performance security posted by 65 Old Bedford Road, LLC; 65 Old Bedford Road, Goldens Bridge, NY 10526; Sheet 4A, Block 11112, Lot 2 (65 Old Bedford Road, LLC; owner of record) established to guarantee the completion of infrastructure associated with the renovation of an existing building into a multi-family dwelling (four (4), 1-bedroom apartments). The subject property consists of approx. 0.72 acres and is located in a Special Character Two-Family Residential (SCR-2F) Zoning District.

A copy of materials pertaining to the requested partial release of the performance security may be inspected at the office of the Planning Board Administrator, 79 Bouton Road, South Salem, New York during regular Planning Board hours. Persons wishing to object to this request should file a notice of objection with the Planning Board together with a statement of the grounds of objection prior to the closing of the Public Hearing. All interested parties are encouraged to attend the Public Hearing and all will be provided an opportunity to be heard.

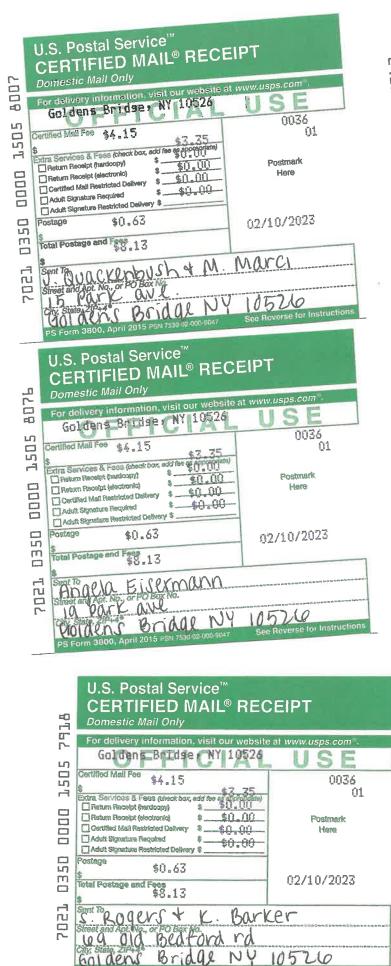
> PLANNING BOARD TOWN OF LEWISBORO By: Janet Andersen Chair

#### Dated: January 19, 2023

The Town of Lewisboro is committed to equal access for all citizens. Anyone needing accommodations to attend or participate in this meeting is encouraged to notify the Administrator to the Planning Board in advance.

1 1	U.S. Postal Service [™] CERTIFIED MAIL [®] REC Domestic Mail Only	EIPT
2 90	For delivery information, visit our website Goldens Bridse NY 10526	ut www.usps.com . USE
0 5 7	Certified Mail Fee \$4.15	0036 01
۳ 000	Extra Services & Fees (check bax, add fee as perspective)       Return Receipt (hardropy)       Return Receipt (electronic)       Return Receipt (electronic)       Conflict Meil Restricted Delivery       Adut Signature Required       Adut Signature Required	Postmark Here
	Postage \$0.63 S Total Postage and Face \$5.13	02/10/2023
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	PS Form 3800, April 2015 PSN 7530-02-000-9047	See Reverse for Instructions

,

















#### U.S. Postal Service™ CERTIFIED MAIL® RECEIPT 2 0 For delivery information, visit our web P-Goldens Bridge NY 10526 ம ertified Mall Fee \$4.15 L) 0138 Extra Services & Fees (check box, add fee as appropriate) E T 02 \$0.00 Return Receipt (electronic) Postmark Certified Mail Restricted Delivery \$0.00 Here Adult Signature Required \$0.00 Adult Signature Restricted Delivery \$ 25 ostage \$0.63 S Total Postage and Fees \$8.13 m 02/10/2023 Server To MICI LAZER + Edith Pervizi Screet and to: No. or PO BOX NO. 54 010 Bedtord rd ц Ц 2 Gordens Bridge NY 105200







# **Ciorsdan Conran**

From: Sent: To: Subject: Ciorsdan Conran Tuesday, February 21, 2023 10:02 AM Ciorsdan Conran FW: 65 Old Bedford Rd.

